

PETROL FILLING STATION, CLOTHALL ROAD, BALDOCK (SG7 6PD)

RESPONSE TO HERTFORDSHIRE COUNTY COUNCIL LLFA COMMENTS, DATED 12 SEPTEMBER 2025

Introduction

This Note has been prepared in response to the comments issued by Hertfordshire County Council Lead Local Flood Authority (LLFA), dated 12/09/2025, regarding the Combined Flood Risk Assessment and Drainage Strategy (FRA & DS) prepared by Civilistix, dated August 2025.

Responses to LLFA Comments

Comment 1

- LLFA's comment:

The hydraulic calculations indicate the inclusion of a soakaway tank (infiltration into the ground); however, the drainage strategy specifies that no infiltration into the ground will occur, and the attenuation tanks are identified as lined permeable paving systems. Please can the clarify whether the proposed discharge is to ground or to a surface water sewer. Should the discharge be directed to ground, a suitable infiltration assessment must be provided in accordance with the requirements of BRE365.

- Response:

The attenuation tanks were initially modelled in Causeway Flow as soakaways with infiltration set to zero. This may have led to confusion. The hydraulic calculations have now been revised to remove all soakaway references to clearly reflect that no infiltration is proposed. The system will discharge to the surface water public sewer, not to ground.

Revised calculations are provided in **Appendix A**.

Comment 2

- LLFA's comment:

Consent must be obtained from the sewerage undertaker for any discharge into their infrastructure, and this consent must explicitly authorise a discharge rate of 2 l/s.

- Response:

Under Section 106 of the Water Industry Act 1991, Anglian Water has a statutory duty to accept a surface water connection unless there is a valid technical reason for refusal. A CCTV survey undertaken following the Drainage Strategy confirmed that the existing site currently discharges into the foul sewer. Redirecting flows to the surface water sewer therefore represents a clear environmental improvement.



Given that the proposed discharge is controlled to 2 l/s, it is considered highly unlikely that Anglian Water would object to the connection. Formal consent (Section 106 Agreement) will be secured with Anglian Water post-planning, at the detailed design stage.

CCTV Survey is provided in **Appendix A**.

Comment 3

- LLFA's comment:

Exceedance of the design 1% AEP rainfall event plus climate change (or failure of the drainage network) must be shown on a drawing, minimising impacts to people and property. This drawing will include proposed external ground levels, finished floor levels and any designed slopes on impermeable surfaces such as car parks.

- Response:

Revised drainage strategy drawing with exceedance routes is provided in **Appendix A**.

Comment 4

- LLFA's comment:

A high-level assessment of how water quantity and water quality will be managed during the construction phase is required.

- Response:

As the project is currently at the planning stage and no contractor has yet been appointed, detailed construction-phase management measures are not available at this time. It is therefore recommended that this requirement be addressed through a planning condition.

Comment 5

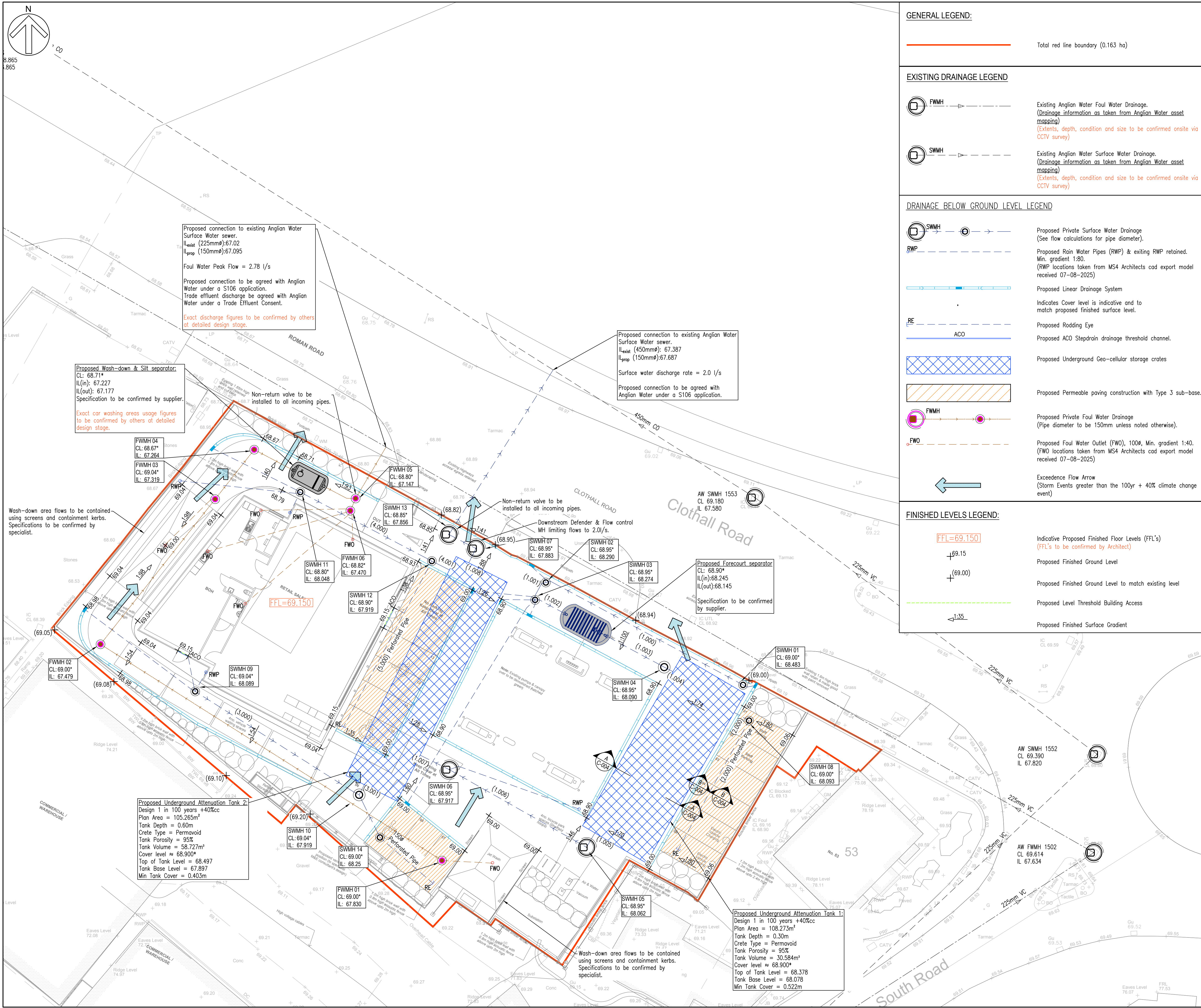
- LLFA's comment:

High level assessment of the maintenance of any SuDS features and structures is provided but who will be adopting these features for the lifetime of the development must be confirmed.

- Response:

The adopting party for the SuDS features has not yet been confirmed at this stage. It is proposed that this requirement be addressed through a planning condition.

Appendix A – Revised Drainage Drawings & Calculations



GENERAL LEGEND:

— Total red line boundary (0.163 ha)

EXISTING DRAINAGE LEGEND

- FWMH Existing Anglian Water Foul Water Drainage. (Drainage information as taken from Anglian Water asset mapping) (Extents, depth, condition and size to be confirmed onsite via CCTV survey)
- SWMH Existing Anglian Water Surface Water Drainage. (Drainage information as taken from Anglian Water asset mapping) (Extents, depth, condition and size to be confirmed onsite via CCTV survey)

DRAINAGE BELOW GROUND LEVEL LEGEND

- SWMH Proposed Private Surface Water Drainage (See flow calculations for pipe diameter).
- RWP Proposed Rain Water Pipes (RWP) & exiting RWP retained. Min. gradient 1:80. (RWP locations taken from MS4 Architects cad export model received 07-08-2025)
- Proposed Linear Drainage System
- ACO Indicates Cover level is indicative and to match proposed finished surface level.
- RE Proposed Rodding Eye
- ACO Proposed ACO Stepdrain drainage threshold channel.
- Proposed Underground Geo-cellular storage crates
- Proposed Permeable paving construction with Type 3 sub-base.
- FWMH Proposed Private Foul Water Drainage (Pipe diameter to be 150mm unless noted otherwise).
- FWO Proposed Foul Water Outlet (FWO), 100Ø, Min. gradient 1:40. (FWO locations taken from MS4 Architects cad export model received 07-08-2025)
- Exceedence Flow Arrow (Storm Events greater than the 100yr + 40% climate change event)

FINISHED LEVELS LEGEND:

- Indicative Proposed Finished Floor Levels (FFL's) (FFL's to be confirmed by Architect)
- Proposed Finished Ground Level
- Proposed Finished Ground Level to match existing level
- Proposed Level Threshold Building Access
- Proposed Finished Surface Gradient

DO NOT SCALE

- GENERAL
- The contractor shall comply with the health and safety requirements as set out in the CDM Regulations
 - All works are to be undertaken in accordance with the Building Regulations and latest relevant British Standards.
 - Conflicting information between this drawing and information given by others must be referred to the engineer before the works commence.
 - The contractor shall, before commencing the works, verify all existing outfall invert levels and site and setting out dimensions. The contractor shall be responsible for the true and proper setting out of the works and for the correctness of the position, levels, dimensions, and alignment of all parts of the works. Any discrepancies are to be reported to the Engineer
 - All products used are to be CE marked in accordance with the Construction Products Directive CPD/89/106/EEC.
 - The contractor shall be responsible for locating all existing utilities prior to commencing construction and protecting all existing services affected by the works.
 - Any unidentified hazards discovered during the progress of works are to be reported immediately to the engineer.
 - This drawing should not be used for setting out
 - All land, ownership and asset boundaries / extents, shown on this plan should be assumed to be indicative and should be legally verified prior to any works commencing

P2	10-10-2025	FR	EXCEEDENCE ROUTES ADDED	FR	DM
P1	12-08-2025	KH	PLANNING ISSUE	FR	DM
REV	DATE	BY	SUMMARY OF CHANGE	CHK	APD

DRAWING STATUS: FOR PLANNING

Allia Future Business Centre, The Guildhall, Cambridge (CB2 3QJ)
Tel: +44 (0)1223 343 277 E: enquiries@civistix.com
www.civistix.com

CLIENT: JP & S Services Ltd

ARCHITECT: MS4 Architects LTD

PROJECT: Petrol Filling Station, Clothall Road, Baldock (SG7 6PD)

DRAWING TITLE: Proposed Private Drainage GA Plan

SCALE @ A1: 1:150	CHECKED / APPROVED BY: FR/DM	DATE: August 2025
PROJECT No: 2-294	DRAWING No: C-002	REV: P2

Nodes

Name	Area (ha)	T of E (mins)	Cover Level (m)	Diameter (mm)	Easting (m)	Northing (m)	Depth (m)
SWMH 01		4.00	69.000	450	525134.859	233565.580	0.517
SWMH 02			68.950	450	525117.771	233574.458	0.660
SWMH 03	0.042	4.00	68.950	450	525116.767	233573.185	0.676
SWMH 04			68.950	600	525128.026	233567.124	0.860
SWMH 05			68.950	1200	525121.299	233551.507	0.888
SWMH 06	0.020	4.00	68.950	1200	525109.408	233558.236	1.033
SWMH 07			68.950	1200	525111.708	233577.407	1.067
SWMH 08	0.007	4.00	69.000	450	525135.380	233562.488	0.907
SWMH 09	0.034	4.00	69.040	450	525087.482	233565.216	0.951
SWMH 10			69.040	600	525101.456	233555.776	1.121
SWMH 11	0.030	4.00	68.800	450	525096.493	233582.289	0.752
SWMH 12	0.008	4.00	68.900	450	525107.891	233576.315	0.981
SWMH 14	0.003	4.00	69.000	450	525103.393	233552.357	1.040
ATT TANK1			68.900	1	525129.100	233566.546	0.822
ATT TANK2			68.900	1	525106.881	233559.493	1.003
Forecourt			68.900	1	525119.314	233571.814	0.755

Links (Input)

Name	US Node	DS Node	Length (m)	ks (mm) / n	US IL (m)	DS IL (m)	Fall (m)	Slope (1:X)	Dia (mm)	T of C (mins)	Rain (mm/hr)
1.000	SWMH 01	SWMH 02	19.257	0.600	68.483	68.290	0.193	99.8	150	4.32	50.0
1.001	SWMH 02	SWMH 03	1.621	0.600	68.290	68.274	0.016	101.3	150	4.35	50.0
1.002	SWMH 03	Forecourt	2.893	0.600	68.274	68.245	0.029	99.8	150	4.39	50.0
1.003	Forecourt	SWMH 04	5.547	0.600	68.145	68.090	0.055	100.9	150	4.49	50.0
1.004	SWMH 04	ATT TANK1	1.220	0.600	68.090	68.078	0.012	101.7	150	4.51	50.0
1.005	ATT TANK1	SWMH 05	1.594	0.600	68.078	68.062	0.016	99.6	150	4.53	50.0
1.006	SWMH 05	SWMH 06	14.513	0.600	68.062	67.917	0.145	100.1	150	4.77	50.0
1.007	SWMH 06	ATT TANK2	1.974	0.600	67.917	67.897	0.020	98.7	150	4.81	50.0
1.008	ATT TANK2	SWMH 07	1.394	0.600	67.897	67.883	0.014	99.6	150	4.83	50.0
2.000	SWMH 08	ATT TANK1	1.470	0.600	68.093	68.078	0.015	98.0	150	4.02	50.0
3.000	SWMH 09	SWMH 10	16.952	0.600	68.089	67.919	0.170	99.7	150	4.28	50.0
3.001	SWMH 10	ATT TANK2	2.223	0.600	67.919	67.897	0.022	101.0	150	4.32	50.0
4.000	SWMH 11	SWMH 12	12.869	0.600	68.048	67.919	0.129	99.8	150	4.21	50.0
4.001	SWMH 12	ATT TANK2	2.161	0.600	67.919	67.897	0.022	98.2	150	4.25	50.0
5.000	SWMH 14	SWMH 10	3.930	0.600	67.960	67.919	0.041	95.8	150	4.06	50.0

Simulation Settings

Rainfall Methodology	FEH-22	Analysis Speed	Normal	Starting Level (m)	
Rainfall Events	Singular	Skip Steady State	x	Check Discharge Rate(s)	x
Summer CV	1.000	Drain Down Time (mins)	3000	Check Discharge Volume	x
Winter CV	1.000	Additional Storage (m³/ha)	0.0		

Storm Durations

15	60	180	360	600	960	2160	4320	7200	10080
30	120	240	480	720	1440	2880	5760	8640	

Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
1	0	0	0
30	40	0	0
100	40	0	0

Node SWMH 07 Online Hydro-Brake® Control

Flap Valve	x	Objective	(HE) Minimise upstream storage
Replaces Downstream Link	✓	Sump Available	✓
Invert Level (m)	67.883	Product Number	CTL-SHE-0073-2000-0599-2000
Design Depth (m)	0.599	Min Outlet Diameter (m)	0.100
Design Flow (l/s)	2.0	Min Node Diameter (mm)	1200

Node ATT TANK1 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	2.0	Invert Level (m)	68.078
Side Inf Coefficient (m/hr)	0.00000	Porosity	0.95	Time to half empty (mins)	488

Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)
0.000	108.1	0.0	0.300	108.1	0.0	0.301	0.0	0.0

Node ATT TANK2 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	2.0	Invert Level (m)	67.897
Side Inf Coefficient (m/hr)	0.00000	Porosity	0.95	Time to half empty (mins)	472

Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)
0.000	105.2	0.0	0.600	105.2	0.0	0.601	0.0	0.0

Results for 1 year Critical Storm Duration. Lowest mass balance: 99.78%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	SWMH 01	1	68.483	0.000	0.0	0.0000	0.0000	OK
15 minute summer	SWMH 02	10	68.333	0.043	0.5	0.0068	0.0000	OK
15 minute summer	SWMH 03	10	68.333	0.059	4.7	0.0093	0.0000	OK
15 minute winter	SWMH 04	9	68.231	0.141	5.2	0.0400	0.0000	OK
180 minute summer	SWMH 05	108	68.089	0.027	1.3	0.0309	0.0000	OK
240 minute summer	SWMH 06	160	67.985	0.068	1.8	0.0764	0.0000	OK
240 minute summer	SWMH 07	160	67.984	0.101	1.9	0.1141	0.0000	OK
15 minute summer	SWMH 08	9	68.122	0.029	0.8	0.0047	0.0000	OK
15 minute summer	SWMH 09	10	68.136	0.047	3.8	0.0075	0.0000	OK
360 minute summer	SWMH 10	232	67.984	0.065	1.2	0.0185	0.0000	OK
15 minute summer	SWMH 11	10	68.092	0.044	3.3	0.0070	0.0000	OK
240 minute summer	SWMH 12	160	67.984	0.065	1.5	0.0104	0.0000	OK
240 minute summer	SWMH 14	160	67.985	0.025	0.1	0.0039	0.0000	OK
180 minute summer	ATT TANK1	108	68.108	0.030	2.3	3.0887	0.0000	OK
240 minute summer	ATT TANK2	160	67.984	0.087	4.6	8.7350	0.0000	OK
15 minute summer	Forecourt	9	68.234	0.089	4.7	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	SWMH 01	1.000	SWMH 02	0.0	0.000	0.000	0.0396	21.6
15 minute summer	SWMH 02	1.001	SWMH 03	-0.5	-0.153	-0.030	0.0085	
15 minute summer	SWMH 03	1.002	Forecourt	4.7	0.788	0.262	0.0171	
15 minute winter	SWMH 04	1.004	ATT TANK1	6.5	1.209	0.369	0.0106	
180 minute summer	SWMH 05	1.006	SWMH 06	1.3	0.365	0.073	0.0656	
240 minute summer	SWMH 06	1.007	ATT TANK2	1.8	0.584	0.099	0.0181	
240 minute summer	SWMH 07	Hydro-Brake®		1.9				
15 minute summer	SWMH 08	2.000	ATT TANK1	0.9	0.826	0.049	0.0019	
15 minute summer	SWMH 09	3.000	SWMH 10	3.8	0.698	0.213	0.0923	
360 minute summer	SWMH 10	3.001	ATT TANK2	1.1	0.452	0.064	0.0200	
15 minute summer	SWMH 11	4.000	SWMH 12	3.3	0.634	0.185	0.0702	
240 minute summer	SWMH 12	4.001	ATT TANK2	1.5	0.506	0.082	0.0195	
240 minute summer	SWMH 14	5.000	SWMH 10	0.1	0.098	0.006	0.0182	
180 minute summer	ATT TANK1	1.005	SWMH 05	1.3	0.548	0.072	0.0037	
240 minute summer	ATT TANK2	1.008	SWMH 07	1.9	0.231	0.107	0.0162	
15 minute summer	Forecourt	1.003	SWMH 04	6.1	0.631	0.344	0.0777	

Results for 30 year +40% CC Critical Storm Duration. Lowest mass balance: 99.78%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	SWMH 01	11	68.593	0.110	2.4	0.0176	0.0000	OK
15 minute summer	SWMH 02	11	68.589	0.299	4.4	0.0476	0.0000	SURCHARGED
15 minute summer	SWMH 03	11	68.588	0.314	29.9	0.0500	0.0000	SURCHARGED
240 minute winter	SWMH 04	232	68.335	0.245	4.9	0.0693	0.0000	SURCHARGED
240 minute winter	SWMH 05	232	68.334	0.272	3.6	0.3078	0.0000	SURCHARGED
240 minute winter	SWMH 06	232	68.334	0.417	5.6	0.4715	0.0000	SURCHARGED
240 minute winter	SWMH 07	232	68.333	0.450	2.1	0.5094	0.0000	OK
240 minute winter	SWMH 08	232	68.334	0.241	0.8	0.0383	0.0000	SURCHARGED
15 minute summer	SWMH 09	10	68.477	0.388	24.2	0.0617	0.0000	SURCHARGED
240 minute winter	SWMH 10	232	68.334	0.415	4.1	0.1174	0.0000	SURCHARGED
15 minute summer	SWMH 11	10	68.355	0.307	21.3	0.0489	0.0000	SURCHARGED
240 minute winter	SWMH 12	232	68.334	0.415	4.3	0.0660	0.0000	SURCHARGED
240 minute winter	SWMH 14	232	68.334	0.374	0.3	0.0594	0.0000	SURCHARGED
240 minute winter	ATT TANK1	232	68.334	0.256	7.2	26.3028	0.0000	SURCHARGED
240 minute winter	ATT TANK2	232	68.334	0.437	12.5	43.6507	0.0000	SURCHARGED
15 minute summer	Forecourt	11	68.464	0.319	26.2	0.0000	0.0000	SURCHARGED

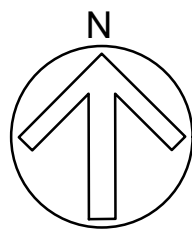
Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	SWMH 01	1.000	SWMH 02	3.6	0.264	0.204	0.3034	
15 minute summer	SWMH 02	1.001	SWMH 03	6.4	0.366	0.365	0.0285	
15 minute summer	SWMH 03	1.002	Forecourt	26.2	1.490	1.476	0.0509	
240 minute winter	SWMH 04	1.004	ATT TANK1	4.8	0.872	0.274	0.0215	
240 minute winter	SWMH 05	1.006	SWMH 06	3.6	0.352	0.203	0.2555	
240 minute winter	SWMH 06	1.007	ATT TANK2	5.4	0.713	0.304	0.0348	
240 minute winter	SWMH 07	Hydro-Brake®		2.0				95.8
240 minute winter	SWMH 08	2.000	ATT TANK1	0.8	0.550	0.043	0.0259	
15 minute summer	SWMH 09	3.000	SWMH 10	23.2	1.316	1.303	0.2984	
240 minute winter	SWMH 10	3.001	ATT TANK2	4.0	0.750	0.227	0.0391	
15 minute summer	SWMH 11	4.000	SWMH 12	20.6	1.171	1.160	0.2266	
240 minute winter	SWMH 12	4.001	ATT TANK2	4.2	0.860	0.237	0.0380	
240 minute winter	SWMH 14	5.000	SWMH 10	0.3	0.108	0.014	0.0692	
240 minute winter	ATT TANK1	1.005	SWMH 05	3.6	0.723	0.204	0.0281	
240 minute winter	ATT TANK2	1.008	SWMH 07	2.1	0.286	0.119	0.0245	
15 minute summer	Forecourt	1.003	SWMH 04	26.4	1.502	1.495	0.0977	

Results for 100 year +40% CC Critical Storm Duration. Lowest mass balance: 99.78%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	SWMH 01	11	68.798	0.315	6.4	0.0501	0.0000	FLOOD RISK
15 minute summer	SWMH 02	11	68.799	0.509	8.7	0.0809	0.0000	FLOOD RISK
15 minute summer	SWMH 03	11	68.798	0.524	37.8	0.0834	0.0000	FLOOD RISK
240 minute winter	SWMH 04	236	68.481	0.391	6.0	0.1108	0.0000	SURCHARGED
240 minute winter	SWMH 05	236	68.480	0.418	3.7	0.4727	0.0000	SURCHARGED
240 minute winter	SWMH 06	232	68.480	0.563	5.9	0.6367	0.0000	SURCHARGED
240 minute winter	SWMH 07	236	68.479	0.596	2.1	0.6745	0.0000	OK
240 minute winter	SWMH 08	232	68.482	0.389	1.0	0.0619	0.0000	SURCHARGED
15 minute summer	SWMH 09	10	68.753	0.664	30.6	0.1056	0.0000	FLOOD RISK
240 minute winter	SWMH 10	236	68.480	0.561	5.1	0.1587	0.0000	SURCHARGED
15 minute summer	SWMH 11	10	68.569	0.521	27.0	0.0828	0.0000	FLOOD RISK
240 minute winter	SWMH 12	236	68.480	0.561	5.4	0.0892	0.0000	SURCHARGED
240 minute winter	SWMH 14	236	68.480	0.520	0.4	0.0827	0.0000	SURCHARGED
240 minute winter	ATT TANK1	228	68.480	0.402	9.0	30.8599	0.0000	SURCHARGED
240 minute winter	ATT TANK2	236	68.480	0.583	13.2	58.2566	0.0000	SURCHARGED
15 minute summer	Forecourt	11	68.611	0.466	32.2	0.0000	0.0000	FLOOD RISK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	SWMH 01	1.000	SWMH 02	-6.4	-0.381	-0.360	0.3390	
15 minute summer	SWMH 02	1.001	SWMH 03	8.8	0.497	0.497	0.0285	
15 minute summer	SWMH 03	1.002	Forecourt	32.2	1.831	1.813	0.0509	
240 minute winter	SWMH 04	1.004	ATT TANK1	5.9	1.036	0.335	0.0215	
240 minute winter	SWMH 05	1.006	SWMH 06	3.7	0.356	0.206	0.2555	
240 minute winter	SWMH 06	1.007	ATT TANK2	5.7	0.624	0.317	0.0348	
240 minute winter	SWMH 07	Hydro-Brake®		2.0				118.8
240 minute winter	SWMH 08	2.000	ATT TANK1	0.9	0.539	0.051	0.0259	
15 minute summer	SWMH 09	3.000	SWMH 10	29.0	1.648	1.632	0.2984	
240 minute winter	SWMH 10	3.001	ATT TANK2	5.0	0.789	0.285	0.0391	
15 minute summer	SWMH 11	4.000	SWMH 12	26.0	1.478	1.463	0.2266	
240 minute winter	SWMH 12	4.001	ATT TANK2	5.3	0.815	0.297	0.0380	
240 minute winter	SWMH 14	5.000	SWMH 10	0.3	0.097	0.018	0.0692	
240 minute winter	ATT TANK1	1.005	SWMH 05	3.7	0.727	0.207	0.0281	
240 minute winter	ATT TANK2	1.008	SWMH 07	2.1	0.299	0.119	0.0245	
15 minute summer	Forecourt	1.003	SWMH 04	32.3	1.835	1.828	0.0977	

Appendix B – CCTV Survey



GENERAL LEGEND:

Total red line boundary (0.346 ha)

CCTV DRAINAGE LEGEND

Existing Private Foul Water Drainage.

Existing Private Surface Water Drainage.

DO NOT SCALE

GENERAL

1. The contractor shall comply with the health and safety requirements as set out in the CDM Regulations
2. All works are to be undertaken in accordance with the Building Regulations and the relevant Building Control Authority
3. Conflicting information between this drawing and information given by others must be referred to the engineer before the works commence.
4. The contractor shall, before commencing the works, verify all existing and smart levels and site and setting out dimensions; the contractor shall be responsible for the true and correct setting out of all works and for the correctness of the positions, levels, dimensions, and alignment of all parts of the construction; the contractor shall be responsible for the correct interpretation of the drawings.
5. All products used are to be CE marked in accordance with the Construction Products Directive (CPD/89/106/EEC).
6. The contractor shall be responsible for locating all existing utilities prior to commencing construction and protecting all existing services and structures affected by the works.
7. Any unidentified hazards discovered during the progress of works are to be reported immediately to the engineer.
8. This drawing should not be used for setting out.
9. All land, ownership and asset boundaries / extents, shown on this plan should be assumed to be indicative and should be verified prior to or during the works.

A	19-09-2025	KH	FIRST ISSUE	FR	IM
REV	DATE	BY	SUMMARY OF CHANGE	CHK	APD

FOR INFORMATION



Allia Future Business Centre, The Guildhall, Cambridge (CB2 3QJ)
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PAWING TITLE

CCTV Drainage Survey Plan

SCALE: AS SHOWN 1:150	CHECKED / APPROVED BY: FR/DM	DATE: September 2025
PROJECT No: 2-294	DRAWING No: C-020	REV: A