

1.0 Introduction

- 1.1 This Technical Note responds to the Lead Local Flood Authorities (LLFA) comments dated 3rd December 2024. A copy of the comments is included in **Appendix A** for ease of reference.

2.0 Comment 1

Construction drawings (General Arrangement drawings for the proposed drainage strategy, schedules, standard detail drawings showing drainage proposals, catchment plans) required to be provided.

- 2.1 The drawings provided with the planning application show the general arrangement of the soakaways with location and size. We have also provided typical sections included construction details through the permeable paving and plot soakaways. We have also provided an impermeable area drawing. The remaining details can be conditioned and provided once detailed design of the site begins.

3.0 Comment 2

Drainage design hydraulic modelling results performed using the proposed site levels as the cover levels for the proposed drainage system. The hydraulic modelling has been performed assuming the uniform cover levels (100m) for all nodes. This replicates the network on a flat ground offering more storage which is higher than on a sloping ground where storage would be practically lesser.

- 3.1.1 Within a system where there is no pipe design and the individual features are not hydraulically linked the cover level is purely arbitrary
- 3.1.2 Each soakaway is modelled from a single node so no assumptions on ground level are made and as long as the difference between the CL and SL reflect the minimum depth of the device then the assessment is accurate. Hence, we have used a generic 100m CL for each node. As we need to update the calculations to account for a change to the CV value we have updated the node CLs to be more reflective of the CL where they are to be located but this will make no difference to the assessment.
- 3.1.3 For the permeable paving design we have undertaken a design for a 10m length only to reflect a typical section of road and applied a base slope of 1/50 as per the calculations. Only one CL can be applied and this doesn't matter. As part of the calculation update we also include a calculation with a base slope of 1/10 to be reflective of all situations.

4.0 Comment 3

The infiltration testing has not been undertaken at the locations where infiltration is proposed. BRE356 infiltration testing at the location where the infiltration features are proposed is required to be provided.

- 4.1.1 Infiltration testing has been completed across the site at different topographical levels to check for any changes to the geology across the site. The test results showed very good infiltration rates at all points of the site with a minimum infiltration rate of 6.2×10^{-6} m/s with most locations recording at least 1×10^{-4} m/s. Infiltration has been designed for a worst case situation using the worst infiltration rate recorded across all tests. Because of the high infiltration rate and the worst case assessment used we would not consider that infiltration testing is needed at all location at this stage. We would contend that completing more that 20 tests across a site of this size would be excessive but if the LLFA require additional infiltration testing this requirement could be conditioned.

5.0 Comment 4

Drainage calculations for events up to and including 1% AEP event plus climate change allowances should be resubmitted with Cv value of 1.

- 5.1.1 The Cv value has been altered to 1 as requested and all calculations and drawings have been updated. New calculations and drawings are enclosed in **Appendix B**.
- 5.1.2 A schedule of the updated plot soakaway sizes is shown below.

Table 5.1: Attenuation Calculation Parameters and Results – Geocellular Soakaways

Parameter	Value
Return Period (years)	100 + 40% Climate Change
Rainfall Parameters	FEH22
Drained Area (ha)	0.144, See Appendix E includes 10% urban creep
Infiltration Rate	6.2×10^{-6} m/s (0.02232 m/hr) – worst case
Factor of Safety	1.5
Porosity	96%
Results	Value (L x W x D)
1	4 x 3 x 1.2m
2	3 x 3 x 1.2m
3	4 x 4 x 1.2m
4	3 x 3 x 1.2m
5	4 x 3 x 1.2m
6	3 x 2 x 0.8m
7	2.5 x 2 x 1.2m
8	3 x 3 x 0.8m
9	5 x 3 x 1.2m
10	3 x 3 x 0.8m
11	4 x 3 x 0.8m
12	4 x 2.5 x 1.2m
13	3 x 3 x 1.2m
14	3 x 3 x 0.8m

6.0 Comment 5

For permeable paving with slopes 3% (1:20) or greater, internal terracing or check dams should be incorporated. Since the calculations/modelling is done using a uniform CL of 100m for all the nodes, actual storage in the permeable paving on a sloping ground has not been estimated here.

- 6.1.1 See our response to comment above. The permeable paving will be designed with check dams and split into small sections to be able to facilitate the gradient of the site.



Mr Alex Howard
Local Planning Authority
North Herts Council
Council Offices
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Letchworth Garden City
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**Lead Local Flood Authority
Growth and Environment
Hertfordshire County Council
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Six Hills Way, Stevenage
HERTFORDSHIRE, SG1 2ST
www.hertfordshire.gov.uk**

Contact Flood Risk Management Team
Email FRMConsultations@hertfordshire.gov.uk

Date 3 December 2024

Dear Mr Howard

RE: 24/01444/FP - Land between Huntsridge and Ashwell House 5 High Street – Ashwell

Thank you for your consultation on the above site, received on 12 November 2024. We have reviewed the application as submitted and wish to make the following comments.

This is a full planning application for the erection of 14 dwellings (including affordable housing) including creation of access from Ashwell Street, footpath link to Lucas Lane, public open space and soft and hard landscaping, biodiversity enhancement and, associated infrastructure on land north of Ashwell Street, Ashwell.

We **maintain our objection** to this planning application in the absence of an acceptable Flood Risk Assessment (FRA) and Drainage Strategy relating to:

- The development not complying with NPPF, PPG or Policy NE 8.

Reason

To prevent flooding in accordance with National Planning Policy Framework paragraphs 173, 175 and 180 by ensuring the satisfactory management of local flood risk, surface water flow paths, storage and disposal of surface water from the site in a range of rainfall events and ensuring the SuDS proposed operates as designed for the lifetime of the development.

We will consider reviewing this objection if the issues addressed in **LLFA technical letter (dated 11 November 2024)** are adequately addressed. The main issues are the following:

1. Construction drawings (General Arrangement drawings for the proposed drainage strategy, schedules, standard detail drawings showing drainage proposals, catchment plans) required to be provided.
2. Drainage design hydraulic modelling results performed using the proposed site levels as the cover levels for the proposed drainage system. The hydraulic modelling has been performed assuming the uniform cover levels (100m) for all nodes. This replicates the network on a flat ground offering more storage which is higher than on a sloping ground where storage would be practically lesser.
3. The infiltration testing has not been undertaken at the locations where infiltration is proposed. BRE356 infiltration testing at the location where the infiltration features are proposed is required to be provided.
4. Drainage calculations for events up to and including 1% AEP event plus climate change allowances should be resubmitted with Cv value of 1.
5. For permeable paving with slopes 3% (1:20) or greater, internal terracing or check dams should be incorporated. Since the calculations/modelling is done using a uniform CL of 100m for all the nodes, actual storage in the permeable paving on a sloping ground has not been estimated here.

Informative

The applicant should seek opportunities to include open above ground SuDS and source control features to address amenity, biodiversity and interception within the drainage strategy elements.

The applicant should provide a construction phase surface water management plan prior to commencement of development. This can be conditioned, assuming the applicant is in agreement with pre-commencement conditions, once the outstanding objection points have been adequately addressed.

For further advice on what we expect to be contained within the FRA to support a planning application, please refer to our Developers Guide and Checklist on our surface water drainage webpage <https://www.hertfordshire.gov.uk/services/recycling-waste-and-environment/water/surface-water-drainage/surface-water-drainage.aspx> this link also includes HCC's policies on SuDS in Hertfordshire.

In December 2022 it was announced FEH rainfall data has been updated to account for additional long term rainfall statistics and new data. As a consequence, the rainfall statistics used for surface water modelling and drainage design has changed. In some areas there is a reduction in comparison to FEH2013 and some places an increase (see [FEH22 - User Guide \(hydrosolutions.co.uk\)](https://www.hydrosolutions.co.uk/FEH22-User-Guide)). Both FEH 2013 and 2023 are currently accepted. For the avoidance of doubt the use of FSR and FEH1999 data has been superseded and therefore, use in rainfall simulations are not accepted.

The applicant should seek opportunity to include open SuDS and source control to address amenity and biodiversity elements within the drainage strategy elements.

Please note if, you the Local Planning Authority review the application and decide to grant planning permission, notify the us (the Lead Local Flood Authority), by email at FRMConsultations@hertfordshire.gov.uk.

Yours sincerely

Raima Prasad
SuDS and Watercourses Support Officer
Growth and Environment

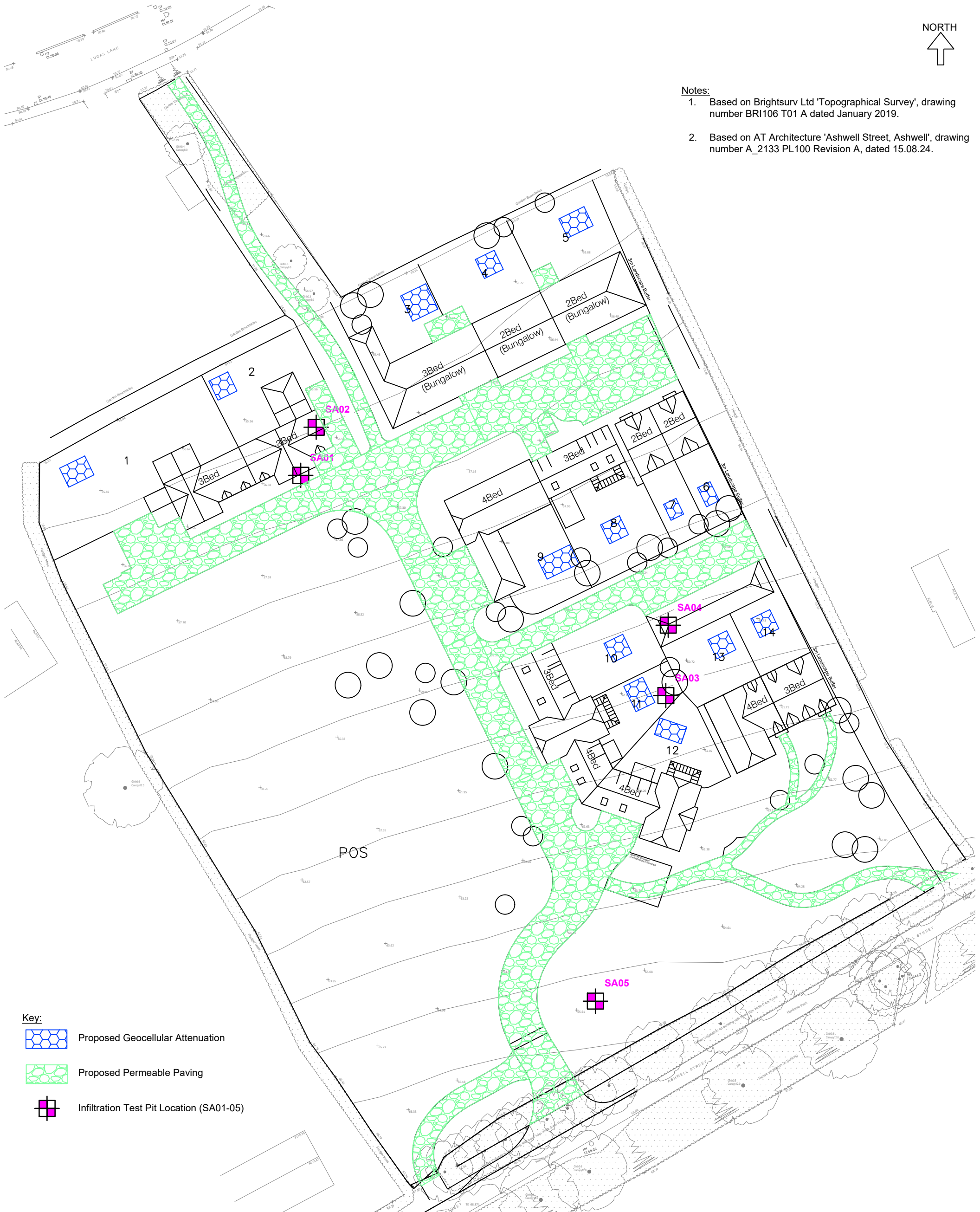
Annex

The following documents have been reviewed, which have been submitted to support the application:

- 190-FRA-02-A-Flood Risk Assessment-Ashwell, by MAC Pre-Planning Engineering, 6 November 2024 and revision D.



Appendix B
Updated drainage design



- Notes:
- 1. Based on Brightsurv Ltd 'Topographical Survey', drawing number BR1106 T01 A dated January 2019.
 - 2. Based on AT Architecture 'Ashwell Street, Ashwell', drawing number A_2133 PL100 Revision A, dated 15.08.24.

Key:

 Proposed Geocellular Attenuation

 Proposed Permeable Paving

 Infiltration Test Pit Location (SA01-05)



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Martin Andrews Consulting Ltd

- Transport Assessments
- Flood Risk Assessments
- Highway Advice
- Access Design
- Drainage Strategies
- Vehicle tracking

Client: Manor Oak Homes		Project: Land off Ashwell Street Ashwell	
Title: Proposed Drainage Strategy		Date: 10.12.24	
		Drw: MJA	
		Chk: MJA	
Drawing No: 190-FRA23		Scale: 1:500	
		Size: A3	
Revision: D			

Design Settings

Rainfall Methodology	FEH-13	Maximum Time of Concentration (mins)	30.00	Preferred Cover Depth (m)	1.200
Return Period (years)	100	Maximum Rainfall (mm/hr)	50.0	Include Intermediate Ground	✓
Additional Flow (%)	0	Minimum Velocity (m/s)	1.00	Enforce best practice design rules	✓
CV	1.000	Connection Type	Level Soffits		
Time of Entry (mins)	5.00	Minimum Backdrop Height (m)	0.200		

Nodes

Name	Area (ha)	T of E (mins)	Cover Level (m)	Node Type	Easting (m)	Northing (m)	Depth (m)
1	0.005	5.00	100.000	Manhole	1.000	1.000	1.000
2	0.005	5.00	100.000	Manhole	2.000	2.000	1.000

Simulation Settings

Rainfall Methodology	FEH-13	Skip Steady State	x	1 year (l/s)	0.0	100 year 360 minute (m³)
Summer CV	1.000	Drain Down Time (mins)	240	30 year (l/s)	0.1	
Winter CV	1.000	Additional Storage (m³/ha)	20.0	100 year (l/s)	0.1	
Analysis Speed	Normal	Check Discharge Rate(s)	✓	Check Discharge Volume	✓	

Storm Durations

15 | 30 | 60 | 120 | 180 | 240 | 360 | 480 | 600 | 720 | 960 | 1440

Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
100	40	0	0

Pre-development Discharge Rate

Site Makeup	Greenfield	Soil Index	1	Growth Factor 30 year	2.55	Q 1 year (l/s)	0.0
Greenfield Method	IH124	SPR	0.10	Growth Factor 100 year	3.56	Q 30 year (l/s)	0.1
Positively Drained Area (ha)	0.284	Region	5	Betterment (%)	0	Q 100 year (l/s)	0.1
SAAR (mm)	569	Growth Factor 1 year	0.87	QBar	0.0		

Pre-development Discharge Volume

Site Makeup	Greenfield	Soil Index	1	Return Period (years)	100	Betterment (%)	0
Greenfield Method	FSR/FEH	SPR	0.10	Climate Change (%)	0	PR	
Positively Drained Area (ha)	0.284	CWI		Storm Duration (mins)	360	Runoff Volume (m³)	

Node 1 Carpark Storage Structure

Base Inf Coefficient (m/hr)	0.02232	Porosity	0.30	Width (m)	5.000	Depth (m)	
Side Inf Coefficient (m/hr)	0.02232	Invert Level (m)	99.000	Length (m)	10.000	Inf Depth (m)	
Safety Factor	2.0	Time to half empty (mins)	489	Slope (1:X)	50.0		

Node 2 Carpark Storage Structure

Base Inf Coefficient (m/hr)	0.02232	Porosity	0.30	Width (m)	5.000	Depth (m)	
Side Inf Coefficient (m/hr)	0.02232	Invert Level (m)	99.000	Length (m)	10.000	Inf Depth (m)	
Safety Factor	2.0	Time to half empty (mins)	446	Slope (1:X)	10.0		

Results for 100 year +40% CC Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
180 minute winter	1	168	99.284	0.284	0.9	2.7975	0.0000	OK
480 minute summer	2	328	99.650	0.650	0.7	3.2794	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	Outflow (l/s)
180 minute winter	1	Infiltration	0.2
480 minute summer	2	Infiltration	0.1



Design Settings

Rainfall Methodology	FEH-13	Maximum Time of Concentration (mins)	30.00	Preferred Cover Depth (m)	1.200
Return Period (years)	100	Maximum Rainfall (mm/hr)	50.0	Include Intermediate Ground	✓
Additional Flow (%)	0	Minimum Velocity (m/s)	1.00	Enforce best practice design rules	✓
CV	1.000	Connection Type	Level Soffits		
Time of Entry (mins)	5.00	Minimum Backdrop Height (m)	0.200		

Nodes

Name	Area (ha)	T of E (mins)	Cover Level (m)	Node Type	Easting (m)	Northing (m)	Depth (m)
1	0.013	5.00	100.000	Manhole	1.000	1.000	44.300
2	0.012	5.00	100.000	Manhole	5.000	5.000	44.400
3	0.018	5.00	100.000	Manhole	10.000	10.000	44.700
4	0.010	5.00	100.000	Manhole	15.000	15.000	44.400
5	0.012	5.00	100.000	Manhole	20.000	20.000	44.400
6	0.005	5.00	100.000	Manhole	25.000	25.000	41.000
7	0.005	5.00	100.000	Manhole	30.000	30.000	41.000
8	0.008	5.00	100.000	Manhole	35.000	35.000	41.000
9	0.015	5.00	100.000	Manhole	40.000	40.000	41.000
10	0.007	5.00	100.000	Manhole	45.000	45.000	39.500
11	0.010	5.00	100.000	Manhole	50.000	50.000	39.000
12	0.012	5.00	100.000	Manhole	55.000	55.000	38.500
13	0.010	5.00	100.000	Manhole	60.000	60.000	39.200
14	0.007	5.00	100.000	Manhole	65.000	65.000	39.200

Simulation Settings

Rainfall Methodology	FEH-13	Skip Steady State	x	1 year (l/s)	0.0	100 year 360 minute (m³)
Summer CV	1.000	Drain Down Time (mins)	240	30 year (l/s)	0.1	
Winter CV	1.000	Additional Storage (m³/ha)	20.0	100 year (l/s)	0.2	
Analysis Speed	Normal	Check Discharge Rate(s)	✓	Check Discharge Volume	✓	

Storm Durations

15	30	60	120	180	240	360	480	600	720	960	1440
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Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
100	40	0	0

Pre-development Discharge Rate

Site Makeup	Greenfield	Soil Index	1	Growth Factor 30 year	2.55	Q 1 year (l/s)	0.0
Greenfield Method	IH124	SPR	0.10	Growth Factor 100 year	3.56	Q 30 year (l/s)	0.1
Positively Drained Area (ha)	0.321	Region	5	Betterment (%)	0	Q 100 year (l/s)	0.2
SAAR (mm)	569	Growth Factor 1 year	0.87	QBar	0.0		

Pre-development Discharge Volume

Site Makeup	Greenfield	Soil Index	1	Return Period (years)	100	Betterment (%)	0
Greenfield Method	FSR/FEH	SPR	0.10	Climate Change (%)	0	PR	
Positively Drained Area (ha)	0.321	CWI		Storm Duration (mins)	360	Runoff Volume (m³)	

Node 1 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.02232	Safety Factor	1.5	Invert Level (m)	55.700
Side Inf Coefficient (m/hr)	0.02232	Porosity	0.96	Time to half empty (mins)	0

Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)
0.000	12.0	12.0	1.200	12.0	28.8	1.201	0.0	28.8

Node 2 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.02232	Safety Factor	1.5	Invert Level (m)	55.600
Side Inf Coefficient (m/hr)	0.02232	Porosity	0.96	Time to half empty (mins)	0

Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)
0.000	9.0	9.0	1.200	9.0	23.4	1.201	0.0	23.4

Node 3 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.02232	Safety Factor	1.5	Invert Level (m)	55.300
Side Inf Coefficient (m/hr)	0.02232	Porosity	0.96	Time to half empty (mins)	0

Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)
0.000	16.0	16.0	1.200	16.0	35.2	1.201	0.0	35.2

Node 4 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.02232	Safety Factor	1.5	Invert Level (m)	55.600
Side Inf Coefficient (m/hr)	0.02232	Porosity	0.96	Time to half empty (mins)	0

Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)
0.000	9.0	9.0	1.200	9.0	23.4	1.201	0.0	23.4

Node 5 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.02232	Safety Factor	1.5	Invert Level (m)	55.600
Side Inf Coefficient (m/hr)	0.02232	Porosity	0.96	Time to half empty (mins)	0

Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)
0.000	12.0	12.0	1.200	12.0	28.8	1.201	0.0	28.8

Node 6 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.02232	Safety Factor	1.5	Invert Level (m)	59.000
Side Inf Coefficient (m/hr)	0.02232	Porosity	0.96	Time to half empty (mins)	0

Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)
0.000	6.0	6.0	0.800	6.0	14.0	0.801	0.0	14.0

Node 7 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.02232	Safety Factor	1.5	Invert Level (m)	59.000
Side Inf Coefficient (m/hr)	0.02232	Porosity	0.96	Time to half empty (mins)	0

Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)
0.000	5.0	5.0	1.200	5.0	15.8	1.201	0.0	15.8

Node 8 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.02232	Safety Factor	1.5	Invert Level (m)	59.000
Side Inf Coefficient (m/hr)	0.02232	Porosity	0.96	Time to half empty (mins)	0

Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)
0.000	9.0	9.0	0.800	9.0	18.6	0.801	0.0	18.6

Node 9 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.02232	Safety Factor	1.5	Invert Level (m)	59.000
Side Inf Coefficient (m/hr)	0.02232	Porosity	0.96	Time to half empty (mins)	0

Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)
0.000	15.0	15.0	1.200	15.0	34.2	1.201	0.0	34.2

Node 10 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.02232	Safety Factor	1.5	Invert Level (m)	60.500
Side Inf Coefficient (m/hr)	0.02232	Porosity	0.96	Time to half empty (mins)	0

Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)
0.000	9.0	9.0	0.800	9.0	18.6	0.801	0.0	18.6

Node 11 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.02232	Safety Factor	1.5	Invert Level (m)	61.000
Side Inf Coefficient (m/hr)	0.02232	Porosity	0.96	Time to half empty (mins)	0

Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)
0.000	12.0	12.0	0.800	12.0	23.2	0.801	0.0	23.2

Node 12 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.02232	Safety Factor	1.5	Invert Level (m)	61.500
Side Inf Coefficient (m/hr)	0.02232	Porosity	0.96	Time to half empty (mins)	0

Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)
0.000	10.0	10.0	1.200	10.0	25.6	1.201	0.0	25.6

Node 13 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.02232	Safety Factor	1.5	Invert Level (m)	60.800
Side Inf Coefficient (m/hr)	0.02232	Porosity	0.96	Time to half empty (mins)	0

Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)
0.000	9.0	9.0	1.200	9.0	19.8	1.201	0.0	19.8

Node 14 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.02232	Safety Factor	1.5	Invert Level (m)	60.800
Side Inf Coefficient (m/hr)	0.02232	Porosity	0.96	Time to half empty (mins)	0

Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)
0.000	9.0	9.0	0.800	9.0	19.8	0.801	0.0	19.8

Results for 100 year +40% CC Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
480 minute summer	1	472	56.664	0.964	1.7	11.1103	0.0000	OK
480 minute winter	2	472	56.791	1.191	1.0	10.2921	0.0000	OK
480 minute winter	3	472	56.311	1.011	1.6	15.5340	0.0000	OK
480 minute winter	4	472	56.587	0.987	0.9	8.5337	0.0000	OK
480 minute winter	5	472	56.478	0.878	1.0	10.1201	0.0000	OK
480 minute summer	6	480	59.765	0.765	0.7	4.4082	0.0000	OK
480 minute summer	7	480	59.931	0.931	0.7	4.4729	0.0000	OK
360 minute winter	8	352	59.778	0.778	0.9	6.7266	0.0000	OK
600 minute summer	9	585	59.885	0.885	1.6	12.7503	0.0000	OK
720 minute winter	10	690	61.160	0.660	0.4	5.7075	0.0000	OK
480 minute winter	11	472	61.723	0.723	0.9	8.3359	0.0000	OK
480 minute winter	12	472	62.565	1.065	1.0	10.2268	0.0000	OK
480 minute winter	13	472	61.806	1.006	0.9	8.6953	0.0000	OK
720 minute winter	14	690	61.452	0.652	0.4	5.6342	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	Outflow (l/s)
480 minute summer	1	Infiltration	0.1
480 minute winter	2	Infiltration	0.1
480 minute winter	3	Infiltration	0.1
480 minute winter	4	Infiltration	0.1
480 minute winter	5	Infiltration	0.1
480 minute summer	6	Infiltration	0.1
480 minute summer	7	Infiltration	0.1
360 minute winter	8	Infiltration	0.1
600 minute summer	9	Infiltration	0.1
720 minute winter	10	Infiltration	0.1
480 minute winter	11	Infiltration	0.1
480 minute winter	12	Infiltration	0.1
480 minute winter	13	Infiltration	0.1
720 minute winter	14	Infiltration	0.1