



Project:		Project No.		Trial Pit:	
Land at Pound Farm, Bedford Road, Ickleford, Hitchin, SG5 3RS		25-267.02		TP1	
Client:		Start:		End:	
Bugler Homes Limited		06/11/2025		06/11/2025	
Method/Plant Used:		Co-ordinates:		Ground Level:	
2.5T Excavator		NT		NT	
Sheet:		Sheet:		Sheet:	
1 of 1		1 of 1		1 of 1	

Description of Strata	Legend	Depth (m bgl) (thickness)	Well Constr.	Samples/Tests									Laboratory Test Details
				Depth	No	Type							
Dark grey brown silty sandy CLAY. Roots to 0.3m. (TOPSOIL)		(0.3) 0.3											
Orange brown silty very sandy gravelly CLAY. Gravel is fine to coarse sub-rounded to sub-angular of flint. (Possibly DIAMICTON)		(0.6) 0.9											
Light brown very silty very sandy gravelly CLAY. Gravel is fine to coarse sub-rounded to sub-angular of flint and chalk. (Possibly DIAMICTON)		(0.5) 1.4											

Dimensions (m)			Water level observations (depths in metres below gl)					
Length	Width	Depth	Date	Water strike	Water level (after 20mins)	Flow	Standing level	Remarks
2.70	0.35	1.40	06/11/2025	-	-	-	-	

Remarks			By
Roots present to 0.3m. No groundwater encountered in trial pit. No visual or olfactory evidence of contamination. Trial pit backfilled with arisings.	Logged	AC	Scale 01:25
	Checked	OB	



TRIAL PIT LOG

Project: Land at Pound Farm, Bedford Road, Ickleford, Hitchin, SG5 3RS				Project No. 25-267.02		Trial Pit: TP2							
Client: Bugler Homes Limited			Start: 06/11/2025		End: 06/11/2025		Sheet: 1 of 1						
Method/Plant Used: 2.5T Excavator		Co-ordinates: NT			Ground Level: NT								
Description of Strata	Legend	Depth (m bgl) (thickness)	Well Cnstr.	Samples/Tests									Laboratory Test Details
				Depth	No	Type							
Dark grey brown silty sandy CLAY. Roots to 0.3m. (TOPSOIL)		(0.3) 0.3											
Orange brown silty sandy gravelly CLAY. gravel is fine to coarse sub-rounded to sub-angular of flint and chalk. (Possibly DIAMICTON)		(0.3) 0.6											
Light brown very silty very sandy gravelly CLAY. Gravel is fine to coarse sub-rounded to sub-angular of flint and chalk. (Possibly DIAMICTON)		(0.4) 1.0											
Light brown very silty very clayey gravelly SAND. Gravel is fine to coarse sub-rounded to sub-angular of flint and chalk. (GLACIOFLUVIAL DEPOSITS)		(0.2) 1.2											
Light brown very silty very sandy gravelly CLAY. Gravel is fine to coarse sub-rounded to sub-angular of flint and chalk. (GLACIOFLUVIAL DEPOSITS)		(0.2) 1.4											

Dimensions (m)			Water level observations (depths in metres below gl)					
Length	Width	Depth	Date	Water strike	Water level (after 20mins)	Flow	Standing level	Remarks
3.30	0.35	1.40	06/11/2025	-	-	-	-	
Remarks Roots present to 0.3m. No groundwater encountered in trial pit. No visual or olfactory evidence of contamination. Trial pit backfilled with arisings.						By		
						Logged	AC	
						Checked	OB	Scale 01:25



TRIAL PIT LOG

Project: Land at Pound Farm, Bedford Road, Ickleford, Hitchin, SG5 3RS				Project No. 25-267.02	Trial Pit: SP1
Client: Bugler Homes Limited			Start: 06/11/2025	End: 07/11/2025	Sheet: 1 of 1
Method/Plant Used: 2.5T Excavator	Co-ordinates: NT			Ground Level: NT	

Description of Strata	Legend	Depth (m bgl) (thickness)	Well Cnstr.	Samples/Tests												Laboratory Test Details
				Depth	No	Type										
Dark grey brown silty sandy CLAY. Roots to 0.3m. (TOPSOIL)		(0.3) 0.3														
Orange brown silty sandy gravelly CLAY. Gravel is fine to coarse sub-rounded to sub-angular of flint. (Possibly DIAMICTON)		(0.5) 0.8														
Orange brown very silty sandy gravelly CLAY. Gravel is fine to coarse sub-rounded to sub-angular of flint. (Possibly DIAMICTON)		(0.2) 1.0														
Orange brown and brown silty gravelly very clayey SAND. Gravel is fine to coarse sub-rounded to sub-angular of flint. (GLACIOFLUVIAL DEPOSITS)		(0.3) 1.3														
Orange brown very silty sandy gravelly CLAY. Gravel is fine to coarse sub-rounded to sub-angular of flint. (GLACIOFLUVIAL DEPOSITS)		(0.2) 1.5														

Dimensions (m)			Water level observations (depths in metres below gl)					
Length	Width	Depth	Date	Water strike	Water level (after 20mins)	Flow	Standing level	Remarks
2.80	0.35	1.50	06/11/2025	-	-	-	-	

Remarks	By	
Roots present to 0.3m. No groundwater encountered in trial pit. No visual or olfactory evidence of contamination. Trial pit backfilled with arisings.	Logged Checked	AC OB Scale 01:25



Dimensions (m)			Water level observations (depths in metres below gl)					
Length	Width	Depth	Date	Water strike	Water level (after 20mins)	Flow	Standing level	Remarks
1.00	0.35	1.00	06/11/2025	-	-	-	-	
						By		
					Logged	AC		
					Checked	OB		Scale 01:25

[illegible]

Dimensions (m)			Water level observations (depths in metres below gl)					
Length	Width	Depth	Date	Water strike	Water level (after 20mins)	Flow	Standing level	Remarks
1.10	0.35	1.00	06/11/2025	-	-	-	-	
						By		
						Logged	AC	
						Checked	OB	Scale 01:25



TP1



TP2



SP1



TP1 arisings



TP2 arisings



SP1 arisings



SP2



SP3



SP2 arisings



SP3 arisings

Soil Infiltration Test

25-267.02: Land at Pound Farm, Bedford Road, Ickleford, Hitchin, SG5 3RS



Test Pit SP1-1

Date: 06/11/2025

Readings Recorded By: OC

Pit Dimensions: 1.5m(l) x 0.35m(w) x 1.5m(d)
Start Water Level: 0.47m

Actual Storage Volume: 0.79 m³
Effective Depth: 1.03 m

Time (mins)	Depth BGL (m)
0	0.470
1	0.480
2	0.480
3	0.490
4	0.500
5	0.510
10	0.520
20	0.540
30	0.560
120	0.660
180	0.700
300	0.770
1294	1.060
1358	1.080
1498	1.120
1634	1.150
1751	1.180

$$\text{Soil infiltration rate, } f = \frac{V_{p75-25}}{a_{p50} \times t_{p75-25}}$$

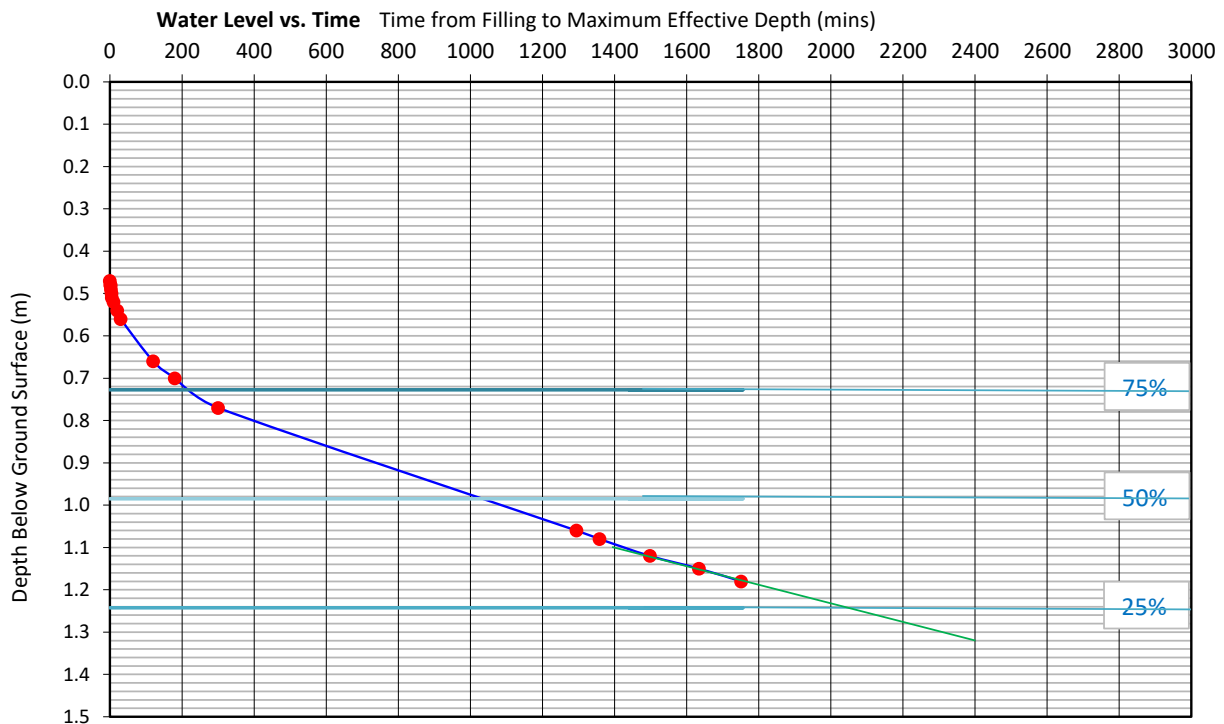
Effective Storage Volume, V : 1.5m x 0.35m x 1.03m
 V : 0.54075 m³

V_{p75-25} : 0.270 m³

Effective Internal Surface Area, a_{p50} : 1.545m² + 0.3605m² + 0.525m²
 a_{p50} : 2.431 m²

Time of water level fall, t_{p75-25} : 2050 - 210 mins
 t_{p75-25} : 1840 mins
 t_{p75-25} : 110400 secs

Soil infiltration rate, f : 0.270375/(2.4305 x 110400)
 f : 1.01E-06 ms⁻¹



Soil Infiltration Test

25-267.02: Land at Pound Farm, Bedford Road, Ickleford, Hitchin, SG5 3RS



Test Pit SP2-1

Date: 06/11/2025

Readings Recorded By: OC

Pit Dimensions: 1m(l) x 0.35m(w) x 1m(d)
Start Water Level: 0.44m

Actual Storage Volume: 0.35 m³
Effective Depth: 0.56 m

Time (mins)	Depth BGL (m)
0	0.440
1	0.450
2	0.460
3	0.470
4	0.480
5	0.490
10	0.520
20	0.540
30	0.560
120	0.640
180	0.670
1228	0.810
1315	0.820
1475	0.840
1689	0.860

$$\text{Soil infiltration rate, } f = \frac{V_{p75-25}}{a_{p50} \times t_{p75-25}}$$

Effective Storage Volume, V : 1m x 0.35m x 0.56m

V : 0.196 m³

V_{p75-25} : 0.098 m³

Effective Internal Surface Area, a_{p50} : 0.56m² + 0.196m² + 0.35m²

a_{p50} : 1.106 m²

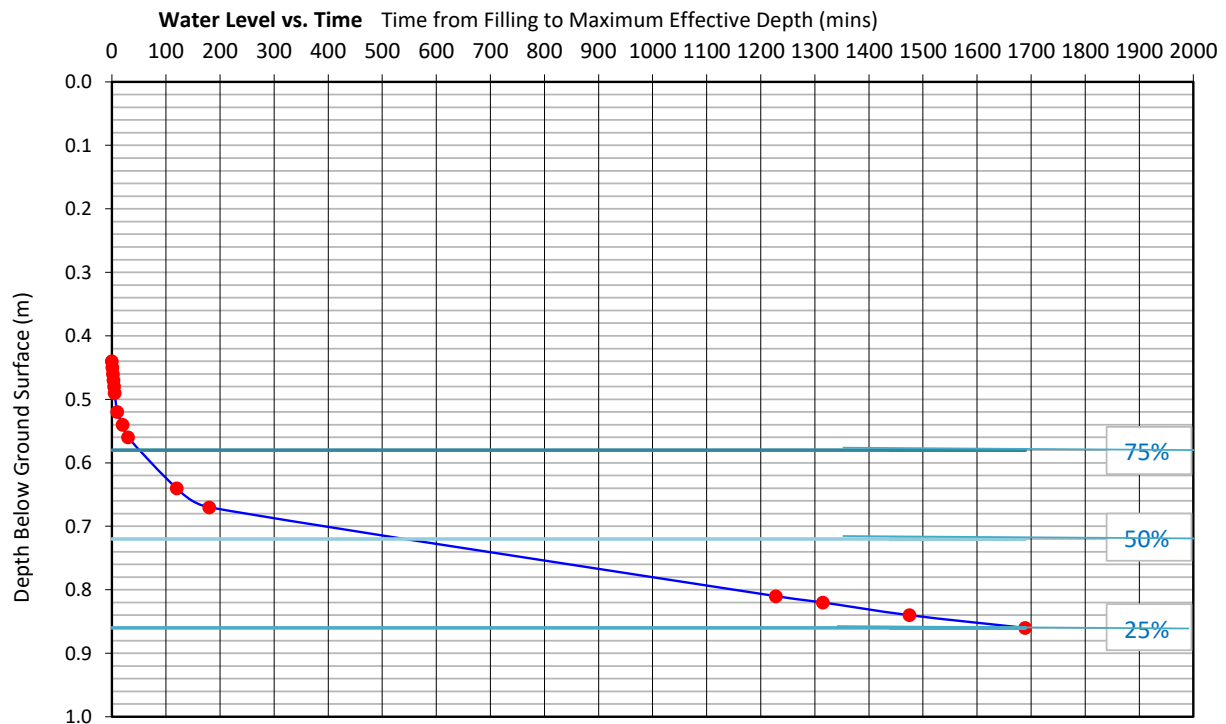
Time of water level fall, t_{p75-25} : 1689 - 50 mins

t_{p75-25} : 1639 mins

t_{p75-25} : 98340 secs

Soil infiltration rate, f : 0.098/(1.106 x 98340)

f : 9.01E-07 ms⁻¹



Soil Infiltration Test

25-267.02: Land at Pound Farm, Bedford Road, Ickleford, Hitchin, SG5 3R



Test Pit SP2-1

Date: 06/11/2025

Readings Recorded By: OC

Pit Dimensions: 1.1m(l) x 0.35m(w) x 1m(d)
Start Water Level: 0.46m

Actual Storage Volume: 0.39 m³
Effective Depth: 0.54 m

Time (mins)	Depth BGL (m)
0	0.460
1	0.470
2	0.480
3	0.490
4	0.500
5	0.510
10	0.530
20	0.550
30	0.570
40	0.590
50	0.600
60	0.610
100	0.670
135	0.700
180	0.720
1215	0.980

$$\text{Soil infiltration rate, } f = \frac{V_{p75-25}}{a_{p50} \times t_{p75-25}}$$

Effective Storage Volume, V : 1.1m x 0.35m x 0.54m
 V : 0.2079 m³

V_{p75-25} : 0.104 m³

Effective Internal Surface Area, a_{p50} : 0.594m² + 0.189m² + 0.385m²

a_{p50} : 1.168 m²

Time of water level fall, t_{p75-25} : 760 - 50 mins

t_{p75-25} : 710 mins

t_{p75-25} : 42600 secs

Soil infiltration rate, f : 0.10395/(1.168 x 42600)

f : 2.09E-06 ms⁻¹



Soil Infiltration Test

25-267.02: Land at Pound Farm, Bedford Road, Ickleford, Hitchin, SG5 3R



Test Pit SP2-1

Date: 07/11/2025

Readings Recorded By: OC

Pit Dimensions: 1.1m(l) x 0.35m(w) x 1m(d)
Start Water Level: 0.4m

Actual Storage Volume: 0.39 m³
Effective Depth: 0.60 m

Time (mins)	Depth BGL (m)
0	0.400
1	0.405
2	0.410
3	0.420
4	0.430
5	0.440
10	0.460
20	0.480
30	0.500
60	0.540
120	0.590
180	0.630
240	0.660

$$\text{Soil infiltration rate, } f = \frac{V_{p75-25}}{a_{p50} \times t_{p75-25}}$$

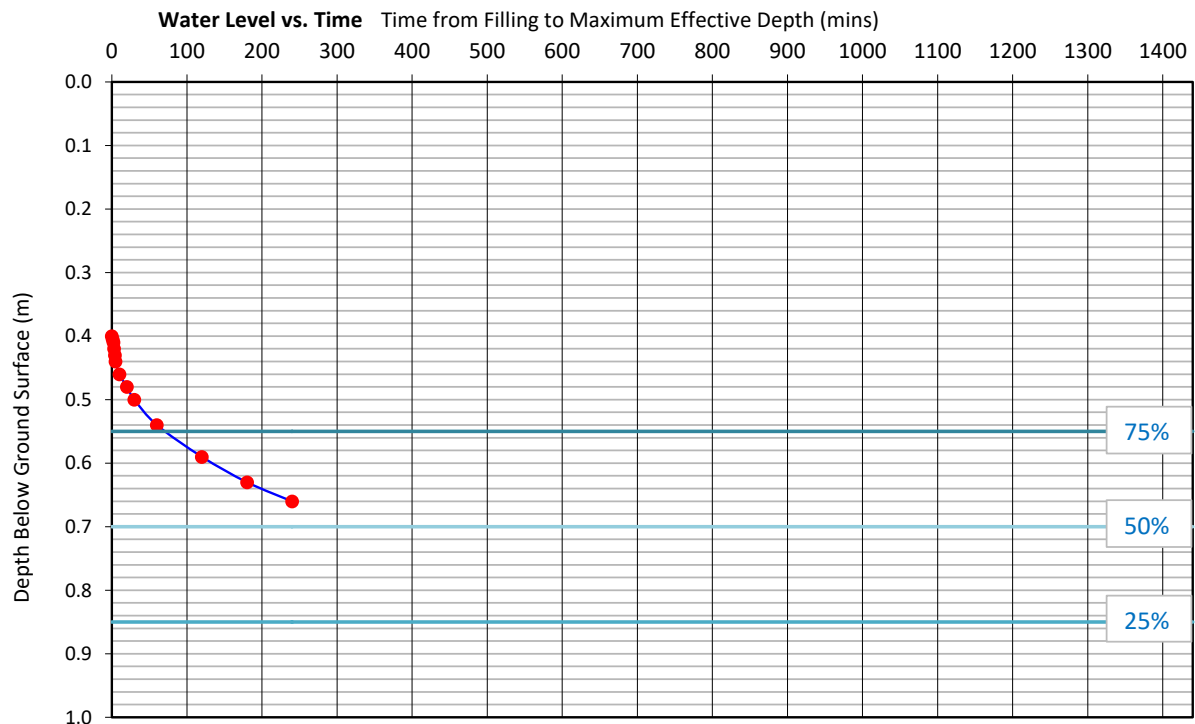
Effective Storage Volume, V : 1.1m x 0.35m x 0.6m
 V : 0.231 m³

V_{p75-25} : 0.116 m³

Effective Internal Surface Area, a_{p50} : 0.66m² + 0.21m² + 0.385m²
 a_{p50} : 1.255 m²

Time of water level fall, t_{p75-25} : - mins
 t_{p75-25} : 0 mins
 t_{p75-25} : 0 secs

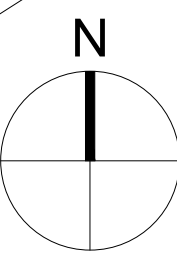
Soil infiltration rate, f : 0.1155/(1.255 x 0)
 f : #DIV/0! ms⁻¹



APPENDIX E



Proposed Masterplan



Revision History & Dates					
Rev	Date	Notes	Amended	Checked	
P1	06/01/20	Initial draft layout	GA	RD	
P2	06/01/20	Layout alterations to 1:2000 scale	GA	RD	
P3	07/01/20	Updated following meeting comments	GA	RD	
P4	08/01/20	Updated following additional meeting comments	GA	RD	

ON ARCHITECTURE
Cornetbury Studio
Logan House, 31 Andrews Close
Cornetbury,
CT12 8P
info@onarchitecture.co.uk
onarchitecture.co.uk
01223 434034

Project Title
**Proposed Residential Development,
Land at Pound Farm, Bedford Road
Bugler Homes**

Drawing Title
**Detailed Application -
Proposed Site Layout Plan
(Ground Floor Plans)**

Rev Number
N/A

Scale
1:500

Date
Jan 2026

Drawn
GA

Checked
RD

Drawing Status
DRAFT

Project No.	Drawing No.	Status	Revision
22.072	101	D	P3

APPENDIX F



Environment Agency Flood Map

Flood map for planning

Your reference	Location (easting/northing)	Created
Unspecified	517462/231620	27 November 2025 11:10

Your selected location is in flood zone 1, an area with a low probability of flooding.

You will need to do a flood risk assessment if your site is **any of the following**:

- bigger than 1 hectare (ha)
- in an area with critical drainage problems as notified by the Environment Agency
- identified as being at increased flood risk in future by the local authority's strategic flood risk assessment
- at risk from other sources of flooding (such as surface water or reservoirs) and its development would increase the vulnerability of its use (such as constructing an office on an undeveloped site or converting a shop to a dwelling)

Notes

The flood map for planning shows river and sea flooding data only. It doesn't include other sources of flooding. It is for use in development planning and flood risk assessments.

This information relates to the selected location and is not specific to any property within it. The map is updated regularly and is correct at the time of printing.

Flood risk data is covered by the Open Government Licence which sets out the terms and conditions for using government data. <https://www.nationalarchives.gov.uk/doc/open-government-licence/version/3>

Use of the address and mapping data is subject to Ordnance Survey public viewing terms under Crown copyright and database rights 2025 AC0000807064. <https://flood-map-for-planning.service.gov.uk/os-terms>



Flood map for planning

Your reference

Unspecified

Location (easting/northing)

517462/231620

Scale

1:2,500

Created

27 Nov 2025 11:10

-  Selected area
-  Flood zone 3
-  Flood zone 2
-  Flood zone 1
-  Flood defence
-  Main river
-  Water storage area

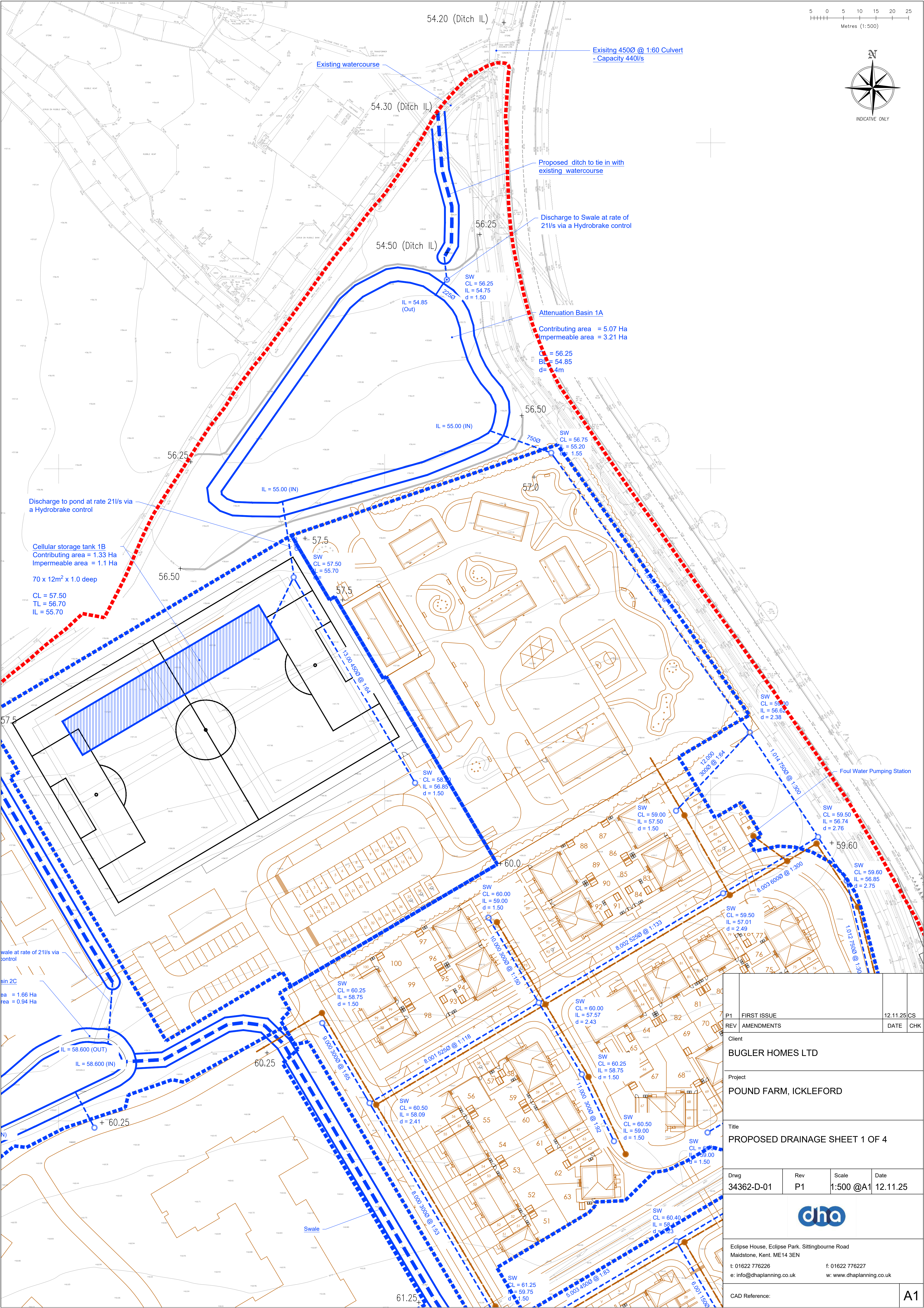


0 20 40 60m

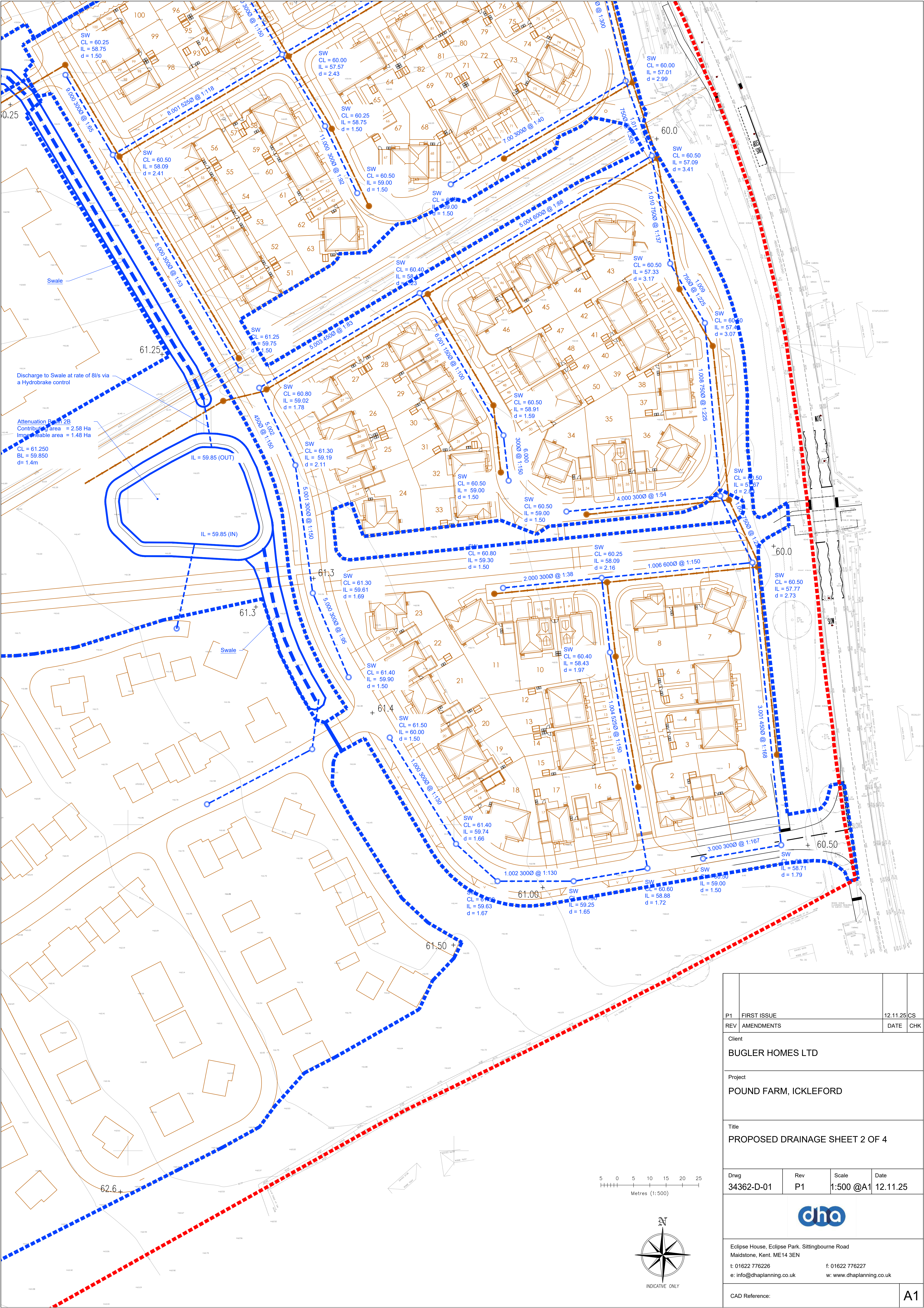
APPENDIX G



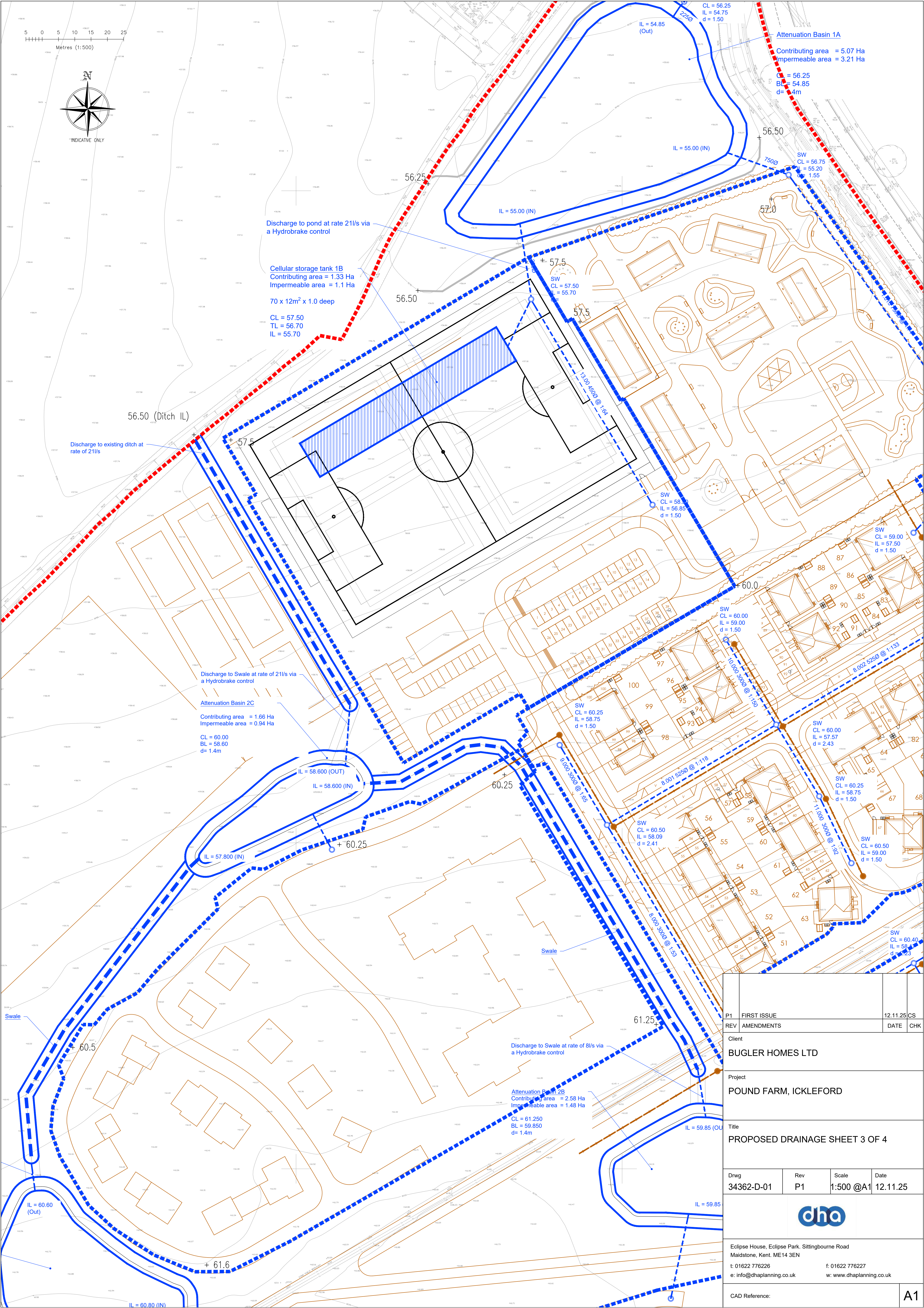
Proposed Drainage Layout and Calculations



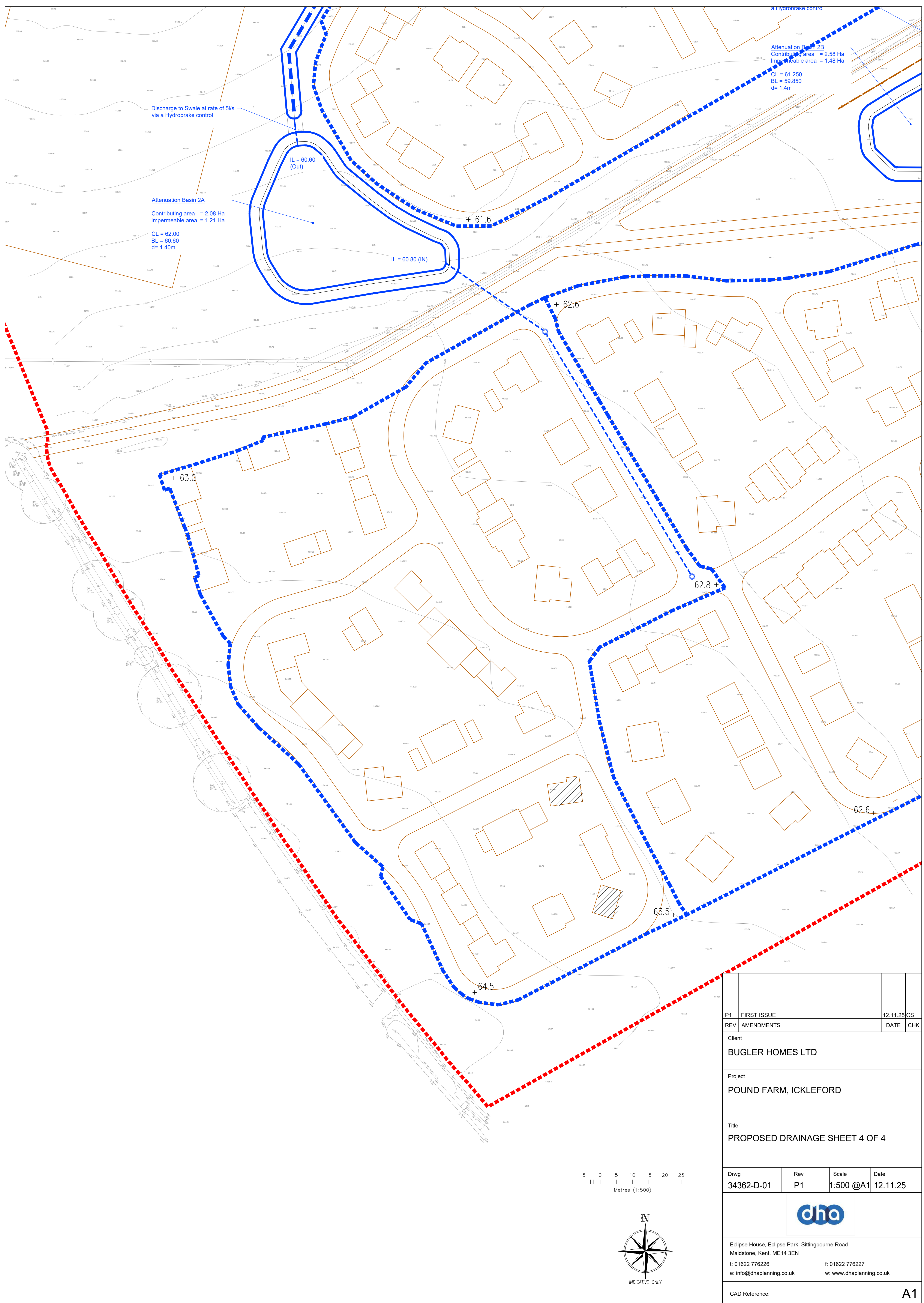
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REV AMENDMENTS		DATE	CHK
Client BUGLER HOMES LTD			
Project POUND FARM, ICKLEFORD			
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Drwg 34362-D-01	Rev P1	Scale 1:500 @A1	Date 12.11.25
Eclipse House, Eclipse Park, Sittingbourne Road Maidstone, Kent, ME14 3EN t: 01622 776226 e: info@dhaplanning.co.uk			f: 01622 776227 w: www.dhaplanning.co.uk
CAD Reference:			A1

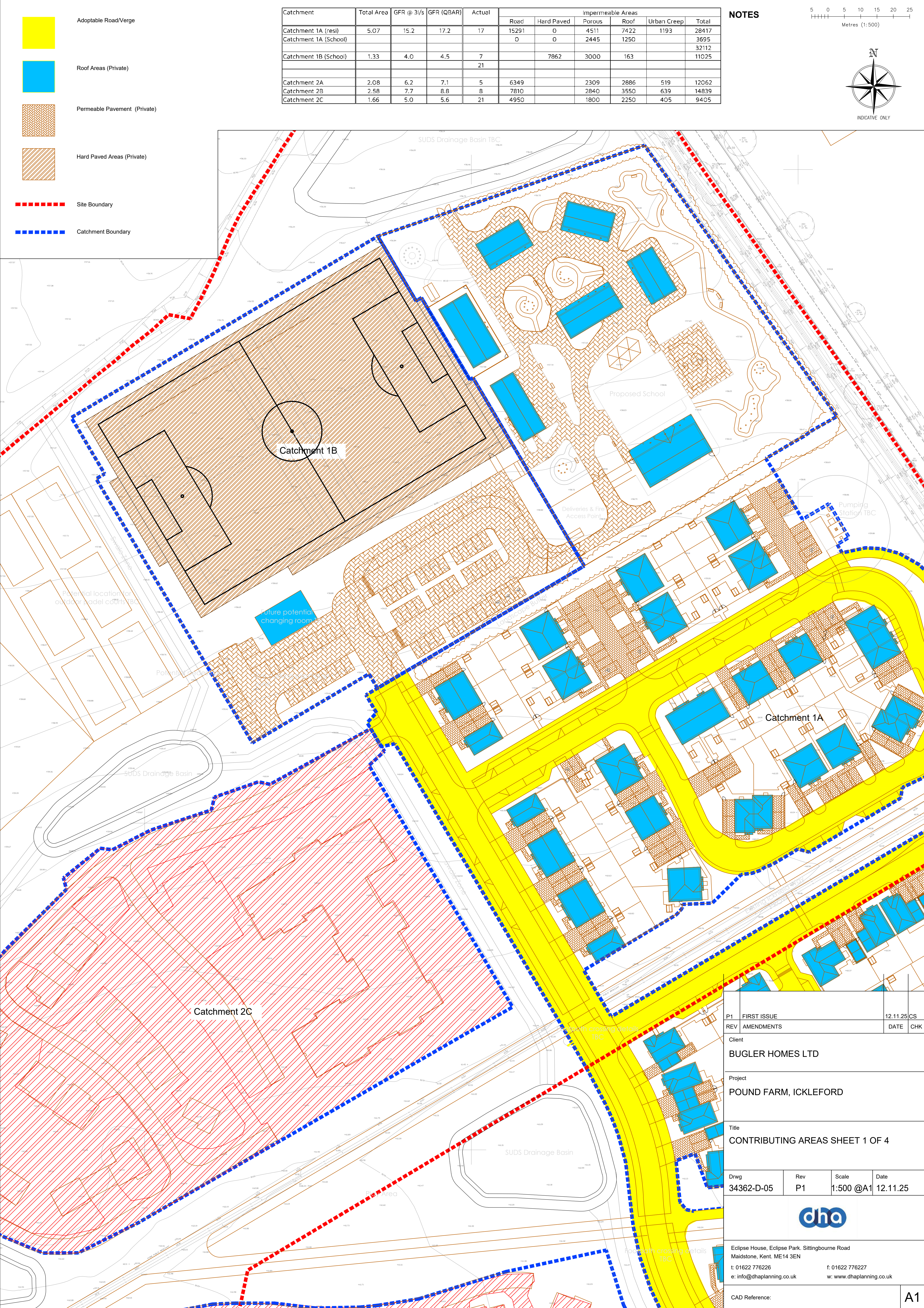


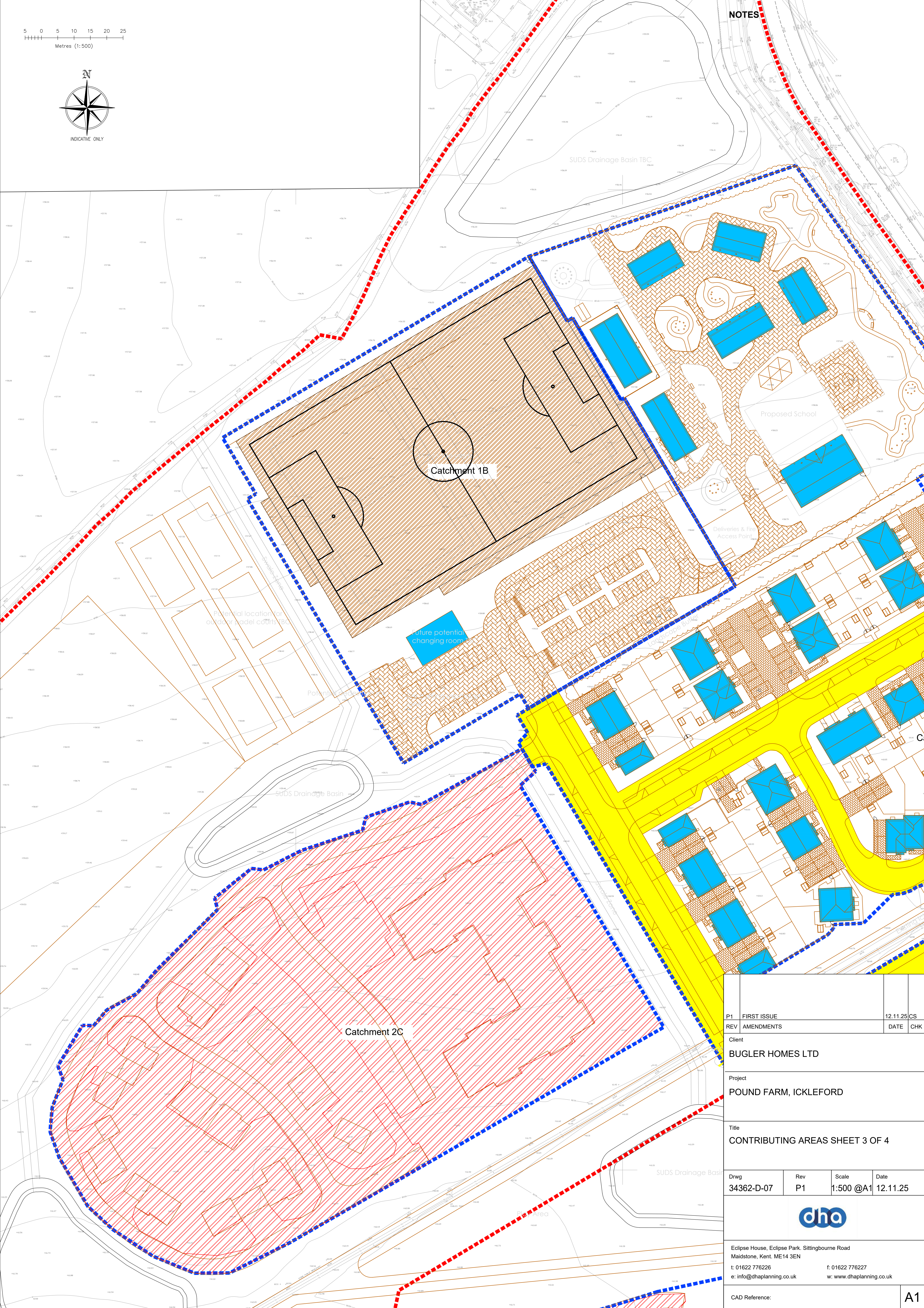
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BUGLER HOMES LTD			
Project			
POUND FARM, ICKLEFORD			
Title			
PROPOSED DRAINAGE SHEET 2 OF 4			
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34362-D-01	P1	1:500 @A1	12.11.25
			
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f: 01622 776227 w: www.dhaplanning.co.uk			
CAD Reference:			A1



P1 FIRST ISSUE		12.11.25	CS
REV AMENDMENTS		DATE	CHK
Client			
BUGLER HOMES LTD			
Project			
POUND FARM, ICKLEFORD			
Title			
PROPOSED DRAINAGE SHEET 3 OF 4			
Drwg	Rev	Scale	Date
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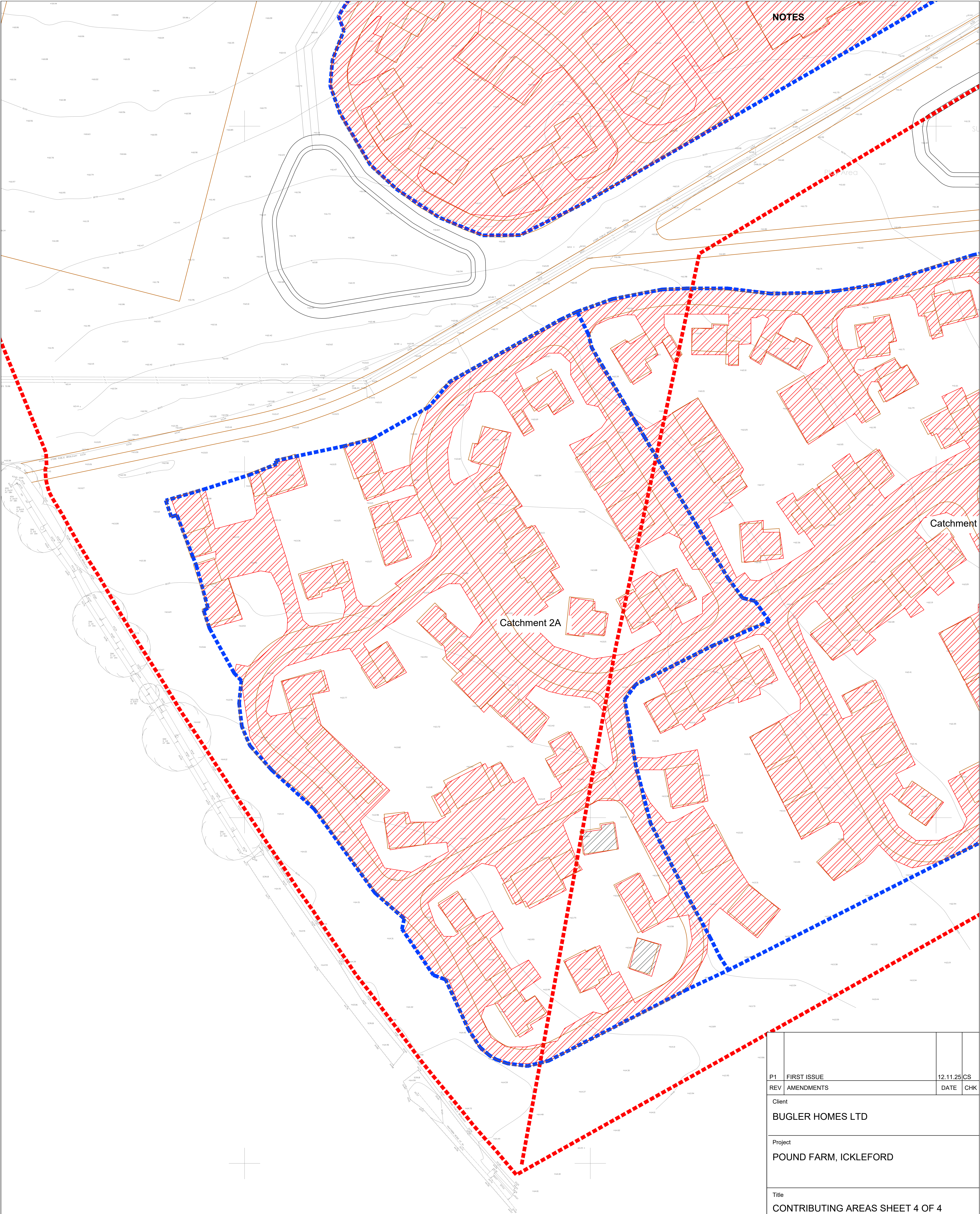


NOTES

Catchment 1B

Catchment 2C

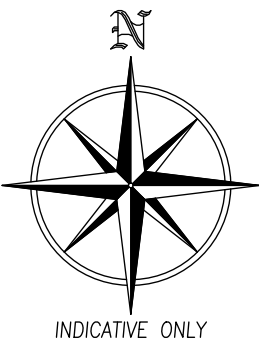
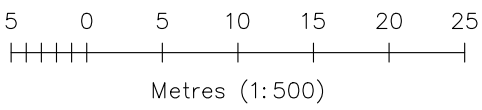
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BUGLER HOMES LTD			
Project			
POUND FARM, ICKLEFORD			
Title			
CONTRIBUTING AREAS SHEET 3 OF 4			
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dha			
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f: 01622 776227 w: www.dhaplanning.co.uk			
CAD Reference:			A1



NOTES

Catchment 2A

Catchment



INDICATIVE ONLY

P1	FIRST ISSUE	12.11.25	CS
REV	AMENDMENTS	DATE	CHK

Client
BUGLER HOMES LTD

Project
POUND FARM, ICKLEFORD

Title
CONTRIBUTING AREAS SHEET 4 OF 4

Drwg	Rev	Scale	Date
34362-D-08	P1	1:500 @A1	12.11.25



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Maidstone, Kent, ME14 3EN
t: 01622 776226
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f: 01622 776227
w: www.dhaplanning.co.uk

CAD Reference:

A1

DHA Transport Ltd		Page 1
Eclipse House Eclipse Park Sittingbourne Road Maidstone ME14 3EN	Pound Farm - Detailed	
Date 13/01/2026 16:14	Designed by Chris	
File sws 08.01.26.MDX	Checked by	

Causeway

Network 2019.1

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FEH Rainfall Model			
Return Period (years)	2	Maximum Time of Concentration (mins)	30
		Foul Sewage (l/s/ha)	0.000
FEH Rainfall Version	1999	Volumetric Runoff Coeff.	0.950
Site Location		PIMP (%)	100
C (1km)	-0.024	Add Flow / Climate Change (%)	0
D1 (1km)	0.324	Minimum Backdrop Height (m)	0.000
D2 (1km)	0.385	Maximum Backdrop Height (m)	0.000
D3 (1km)	0.273	Min Design Depth for Optimisation (m)	1.200
E (1km)	0.310	Min Vel for Auto Design only (m/s)	1.00
F (1km)	2.515	Min Slope for Optimisation (1:X)	350
Maximum Rainfall (mm/hr)	0		

Designed with Level Soffits

Time Area Diagram for Storm

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	1.803	4-8	2.415	8-12	0.104

Total Area Contributing (ha) = 4.322

Total Pipe Volume (m³) = 357.597

Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	38.400	0.256	150.0	0.120	4.00	0.0	0.600	o	300	Pipe/Conduit	
1.001	17.100	0.114	150.0	0.026	0.00	0.0	0.600	o	300	Pipe/Conduit	
1.002	29.900	0.230	130.0	0.050	0.00	0.0	0.600	o	300	Pipe/Conduit	
1.003	23.900	0.300	79.7	0.046	0.00	0.0	0.600	o	450	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	E I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	0.00	4.50	60.000	0.120	0.0	0.0	0.0	1.28	90.6	0.0
1.001	0.00	4.72	59.744	0.146	0.0	0.0	0.0	1.28	90.6	0.0
1.002	0.00	5.08	59.630	0.196	0.0	0.0	0.0	1.38	97.4	0.0
1.003	0.00	5.26	59.250	0.242	0.0	0.0	0.0	2.28	362.5	0.0

DHA Transport Ltd										Page 2		
Eclipse House Eclipse Park Sittingbourne Road Maidstone ME14 3EN						Pound Farm - Detailed						
Date 13/01/2026 16:14 File sws 08.01.26.MDX						Designed by Chris Checked by						
Causeway						Network 2019.1						
Network Design Table for Storm												
PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section	Type	Auto Design
1.004	67.250	0.448	150.1	0.130	0.00	0.0	0.600	o	525	Pipe/Conduit		
1.005	23.600	0.157	150.0	0.028	0.00	0.0	0.600	o	525	Pipe/Conduit		
2.000	30.400	0.806	37.7	0.063	4.00	0.0	0.600	o	300	Pipe/Conduit		
1.006	40.500	0.270	150.0	0.050	0.00	0.0	0.600	o	600	Pipe/Conduit		
3.000	24.300	0.145	167.6	0.122	4.00	0.0	0.600	o	300	Pipe/Conduit		
3.001	80.500	0.479	168.1	0.075	0.00	0.0	0.600	o	450	Pipe/Conduit		
1.007	23.300	0.104	224.0	0.051	0.00	0.0	0.600	o	750	Pipe/Conduit		
4.000	47.400	0.880	53.9	0.101	4.00	0.0	0.600	o	300	Pipe/Conduit		
1.008	53.250	0.237	224.7	0.118	0.00	0.0	0.600	o	750	Pipe/Conduit		
1.009	23.100	0.103	224.3	0.054	0.00	0.0	0.600	o	750	Pipe/Conduit		
1.010	33.100	0.242	136.8	0.071	0.00	0.0	0.600	o	750	Pipe/Conduit		
5.000	28.000	0.295	94.9	0.100	4.00	0.0	0.600	o	300	Pipe/Conduit		
5.001	39.600	0.264	150.0	0.082	0.00	0.0	0.600	o	300	Pipe/Conduit		
5.002	26.200	0.175	150.0	0.087	0.00	0.0	0.600	o	450	Pipe/Conduit		
5.003	57.100	0.692	82.5	0.062	0.00	0.0	0.600	o	450	Pipe/Conduit		
Network Results Table												
PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)		
1.004	0.00	5.87	58.875	0.372	0.0	0.0	0.0	1.83	395.3	0.0		
1.005	0.00	6.09	58.427	0.400	0.0	0.0	0.0	1.83	395.4	0.0		
2.000	0.00	4.20	59.300	0.063	0.0	0.0	0.0	2.57	181.5	0.0		
1.006	0.00	6.43	58.194	0.513	0.0	0.0	0.0	1.99	561.6	0.0		
3.000	0.00	4.33	59.000	0.122	0.0	0.0	0.0	1.21	85.6	0.0		
3.001	0.00	5.19	58.705	0.197	0.0	0.0	0.0	1.57	249.0	0.0		
1.007	0.00	6.64	57.774	0.761	0.0	0.0	0.0	1.87	824.2	0.0		
4.000	0.00	4.37	59.000	0.101	0.0	0.0	0.0	2.15	151.8	0.0		
1.008	0.00	7.11	57.670	0.980	0.0	0.0	0.0	1.86	823.0	0.0		
1.009	0.00	7.32	57.433	1.034	0.0	0.0	0.0	1.86	823.7	0.0		
1.010	0.00	7.55	57.330	1.105	0.0	0.0	0.0	2.39	1056.4	0.0		
5.000	0.00	4.29	59.900	0.100	0.0	0.0	0.0	1.61	114.1	0.0		
5.001	0.00	4.80	59.605	0.182	0.0	0.0	0.0	1.28	90.6	0.0		
5.002	0.00	5.07	59.191	0.269	0.0	0.0	0.0	1.66	263.6	0.0		
5.003	0.00	5.49	59.016	0.331	0.0	0.0	0.0	2.24	356.2	0.0		
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<u>Network Design Table for Storm</u>												
PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section	Type	Auto Design
6.000	14.000	0.093	150.0	0.044	4.00	0.0	0.600	o	300	Pipe/Conduit		
6.001	50.700	0.338	150.0	0.134	0.00	0.0	0.600	o	300	Pipe/Conduit		
5.004	82.750	0.936	88.4	0.129	0.00	0.0	0.600	o	600	Pipe/Conduit		
1.011	24.300	0.081	300.0	0.000	0.00	0.0	0.600	o	750	Pipe/Conduit		
7.000	62.000	1.543	40.2	0.148	4.00	0.0	0.600	o	300	Pipe/Conduit		
1.012	46.800	0.156	300.0	0.070	0.00	0.0	0.600	o	750	Pipe/Conduit		
1.013	16.000	0.108	148.1	0.000	0.00	0.0	0.600	o	750	Pipe/Conduit		
8.000	76.700	1.440	53.3	0.200	4.00	0.0	0.600	o	300	Pipe/Conduit		
9.000	28.500	0.440	64.8	0.097	4.00	0.0	0.600	o	300	Pipe/Conduit		
8.001	60.300	0.511	118.0	0.086	0.00	0.0	0.600	o	525	Pipe/Conduit		
10.000	30.200	0.201	150.2	0.111	4.00	0.0	0.600	o	300	Pipe/Conduit		
11.000	22.800	0.250	91.2	0.076	4.00	0.0	0.600	o	300	Pipe/Conduit		
<u>Network Results Table</u>												
PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)		
6.000	0.00	4.18	59.000	0.044	0.0	0.0	0.0	1.28	90.6	0.0		
6.001	0.00	4.84	58.907	0.178	0.0	0.0	0.0	1.28	90.6	0.0		
5.004	0.00	6.02	58.174	0.638	0.0	0.0	0.0	2.59	732.6	0.0		
1.011	0.00	7.80	57.088	1.743	0.0	0.0	0.0	1.61	711.5	0.0		
7.000	0.00	4.42	59.000	0.148	0.0	0.0	0.0	2.49	175.8	0.0		
1.012	0.00	8.28	57.007	1.961	0.0	0.0	0.0	1.61	711.5	0.0		
1.013	0.00	8.40	56.851	1.961	0.0	0.0	0.0	2.30	1014.8	0.0		
8.000	0.00	4.59	59.750	0.200	0.0	0.0	0.0	2.16	152.6	0.0		
9.000	0.00	4.24	58.750	0.097	0.0	0.0	0.0	1.96	138.3	0.0		
8.001	0.00	5.08	58.085	0.383	0.0	0.0	0.0	2.06	446.2	0.0		
10.000	0.00	4.39	58.500	0.111	0.0	0.0	0.0	1.28	90.5	0.0		
11.000	0.00	4.23	59.000	0.076	0.0	0.0	0.0	1.65	116.4	0.0		
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Network Design Table for Storm											
PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
11.001	25.600	0.951	26.9	0.048	0.00	0.0	0.600	o	300	Pipe/Conduit	
8.002	65.800	0.493	133.5	0.191	0.00	0.0	0.600	o	525	Pipe/Conduit	
8.003	33.900	0.113	300.0	0.020	0.00	0.0	0.600	o	600	Pipe/Conduit	
1.014	38.300	0.128	300.0	0.000	0.00	0.0	0.600	o	750	Pipe/Conduit	
12.000	33.000	0.430	76.7	0.062	4.00	0.0	0.600	o	300	Pipe/Conduit	
1.015	105.200	1.415	74.3	0.370	0.00	0.0	0.600	o	750	Pipe/Conduit	
1.016	20.000	0.200	100.0	0.000	0.00	0.0	0.600	o	750	Pipe/Conduit	
13.000	73.200	1.150	63.7	0.550	4.00	0.0	0.600	o	450	Pipe/Conduit	
14.000	73.200	1.150	63.7	0.550	4.00	0.0	0.600	o	450	Pipe/Conduit	
13.001	23.000	0.700	32.9	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
1.017	12.000	0.080	150.0	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
Network Results Table											
PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)	
11.001	0.00	4.37	58.750	0.124	0.0	0.0	0.0	3.04	215.0	0.0	
8.002	0.00	5.65	57.574	0.809	0.0	0.0	0.0	1.94	419.4	0.0	
8.003	0.00	6.05	57.006	0.829	0.0	0.0	0.0	1.40	396.0	0.0	
1.014	0.00	8.80	56.743	2.790	0.0	0.0	0.0	1.61	711.5	0.0	
12.000	0.00	4.31	57.500	0.062	0.0	0.0	0.0	1.80	127.0	0.0	
1.015	0.00	9.34	56.615	3.222	0.0	0.0	0.0	3.25	1434.9	0.0	
1.016	0.00	9.46	55.200	3.222	0.0	0.0	0.0	2.80	1236.4	0.0	
13.000	0.00	4.48	56.850	0.550	0.0	0.0	0.0	2.55	405.8	0.0	
14.000	0.00	4.48	56.850	0.550	0.0	0.0	0.0	2.55	405.8	0.0	
13.001	0.00	4.65	55.700	1.100	0.0	0.0	0.0	2.29	91.1	0.0	
1.017	0.00	9.64	54.850	4.322	0.0	0.0	0.0	1.07	42.4	0.0	
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Manhole Schedules for Storm

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam., L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	Pipes In PN	Invert Level (m)	Diameter (mm)	Backdrop (mm)
1	61.500	1.500	Open Manhole	1200	1.000	60.000	300				
2	61.400	1.656	Open Manhole	1200	1.001	59.744	300	1.000	59.744	300	
3	61.300	1.670	Open Manhole	1200	1.002	59.630	300	1.001	59.630	300	
4	60.900	1.650	Open Manhole	1350	1.003	59.250	450	1.002	59.400	300	
5	60.600	1.725	Open Manhole	1500	1.004	58.875	525	1.003	58.950	450	
6	60.400	1.973	Open Manhole	1500	1.005	58.427	525	1.004	58.427	525	
7	60.800	1.500	Open Manhole	1200	2.000	59.300	300				
8	60.250	2.056	Open Manhole	1500	1.006	58.194	600	1.005	58.270	525	1
								2.000	58.494	300	
9	60.500	1.500	Open Manhole	1200	3.000	59.000	300				
10	60.500	1.795	Open Manhole	1350	3.001	58.705	450	3.000	58.855	300	
11	60.500	2.726	Open Manhole	1800	1.007	57.774	750	1.006	57.924	600	
								3.001	58.226	450	152
12	60.500	1.500	Open Manhole	1200	4.000	59.000	300				
13	60.500	2.830	Open Manhole	1800	1.008	57.670	750	1.007	57.670	750	
								4.000	58.120	300	
14	60.500	3.067	Open Manhole	1800	1.009	57.433	750	1.008	57.433	750	
15	60.500	3.170	Open Manhole	1800	1.010	57.330	750	1.009	57.330	750	
16	61.400	1.500	Open Manhole	1200	5.000	59.900	300				
17	61.300	1.695	Open Manhole	1200	5.001	59.605	300	5.000	59.605	300	
18	61.000	1.809	Open Manhole	1350	5.002	59.191	450	5.001	59.341	300	
19	60.800	1.784	Open Manhole	1350	5.003	59.016	450	5.002	59.016	450	
20	60.500	1.500	Open Manhole	1200	6.000	59.000	300				
21	60.500	1.593	Open Manhole	1200	6.001	58.907	300	6.000	58.907	300	
22	60.400	2.226	Open Manhole	1500	5.004	58.174	600	5.003	58.324	450	
								6.001	58.569	300	94
23	60.500	3.412	Open Manhole	1800	1.011	57.088	750	1.010	57.088	750	
								5.004	57.238	600	
24	60.500	1.500	Open Manhole	1200	7.000	59.000	300				
25	60.000	2.993	Open Manhole	1800	1.012	57.007	750	1.011	57.007	750	
								7.000	57.457	300	
26	59.600	2.749	Open Manhole	1800	1.013	56.851	750	1.012	56.851	750	
27	61.250	1.500	Open Manhole	1200	8.000	59.750	300				
28	60.250	1.500	Open Manhole	1200	9.000	58.750	300				
29	60.500	2.415	Open Manhole	1500	8.001	58.085	525	8.000	58.310	300	
								9.000	58.310	300	
30	60.000	1.500	Open Manhole	1200	10.000	58.500	300				

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Manhole Schedules for Storm

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam., L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Backdrop (mm)
31	60.500	1.500	Open Manhole	1200	11.000	59.000	300				
32	60.250	1.500	Open Manhole	1200	11.001	58.750	300	11.000	58.750	300	
33	59.900	2.326	Open Manhole	1500	8.002	57.574	525	8.001	57.574	525	
								10.000	58.299	300	500
								11.001	57.799	300	
35	59.500	2.494	Open Manhole	1500	8.003	57.006	600	8.002	57.081	525	
36	59.500	2.757	Open Manhole	1800	1.014	56.743	750	1.013	56.743	750	
								8.003	56.893	600	
37	59.000	1.500	Open Manhole	1200	12.000	57.500	300				
37	59.000	2.385	Open Manhole	1800	1.015	56.615	750	1.014	56.615	750	
								12.000	57.070	300	5
38	56.750	1.550	Open Manhole	1800	1.016	55.200	750	1.015	55.200	750	
39	58.500	1.650	Open Manhole	1350	13.000	56.850	450				
40	58.500	1.650	Open Manhole	1350	14.000	56.850	450				
40	57.500	1.800	Open Manhole	1350	13.001	55.700	225	13.000	55.700	450	
								14.000	55.700	450	
41	56.250	1.400	Open Manhole	1800	1.017	54.850	225	1.016	55.000	750	675
								13.001	55.000	225	150
	56.250	1.480	Open Manhole	0		OUTFALL		1.017	54.770	225	

No coordinates have been specified, layout information cannot be produced.

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PIPELINE SCHEDULES for Storm								
Upstream Manhole								
PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
1.000	o	300	1	61.500	60.000	1.200	Open Manhole	1200
1.001	o	300	2	61.400	59.744	1.356	Open Manhole	1200
1.002	o	300	3	61.300	59.630	1.370	Open Manhole	1200
1.003	o	450	4	60.900	59.250	1.200	Open Manhole	1350
1.004	o	525	5	60.600	58.875	1.200	Open Manhole	1500
1.005	o	525	6	60.400	58.427	1.448	Open Manhole	1500
2.000	o	300	7	60.800	59.300	1.200	Open Manhole	1200
1.006	o	600	8	60.250	58.194	1.456	Open Manhole	1500
3.000	o	300	9	60.500	59.000	1.200	Open Manhole	1200
3.001	o	450	10	60.500	58.705	1.345	Open Manhole	1350
1.007	o	750	11	60.500	57.774	1.976	Open Manhole	1800
4.000	o	300	12	60.500	59.000	1.200	Open Manhole	1200
1.008	o	750	13	60.500	57.670	2.080	Open Manhole	1800
1.009	o	750	14	60.500	57.433	2.317	Open Manhole	1800
Downstream Manhole								
PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
1.000	38.400	150.0	2	61.400	59.744	1.356	Open Manhole	1200
1.001	17.100	150.0	3	61.300	59.630	1.370	Open Manhole	1200
1.002	29.900	130.0	4	60.900	59.400	1.200	Open Manhole	1350
1.003	23.900	79.7	5	60.600	58.950	1.200	Open Manhole	1500
1.004	67.250	150.1	6	60.400	58.427	1.448	Open Manhole	1500
1.005	23.600	150.0	8	60.250	58.270	1.455	Open Manhole	1500
2.000	30.400	37.7	8	60.250	58.494	1.456	Open Manhole	1500
1.006	40.500	150.0	11	60.500	57.924	1.976	Open Manhole	1800
3.000	24.300	167.6	10	60.500	58.855	1.345	Open Manhole	1350
3.001	80.500	168.1	11	60.500	58.226	1.824	Open Manhole	1800
1.007	23.300	224.0	13	60.500	57.670	2.080	Open Manhole	1800
4.000	47.400	53.9	13	60.500	58.120	2.080	Open Manhole	1800
1.008	53.250	224.7	14	60.500	57.433	2.317	Open Manhole	1800
1.009	23.100	224.3	15	60.500	57.330	2.420	Open Manhole	1800
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Causeway		Network 2019.1																																																																																																																																																																																																																																																													
PIPELINE SCHEDULES for Storm Upstream Manhole <table><thead><tr><th>PN</th><th>Hyd Sect</th><th>Diam (mm)</th><th>MH Name</th><th>C.Level (m)</th><th>I.Level (m)</th><th>D.Depth (m)</th><th>MH Connection</th><th>MH DIAM., L*W (mm)</th></tr></thead><tbody><tr><td>1.010</td><td>o</td><td>750</td><td>15</td><td>60.500</td><td>57.330</td><td>2.420</td><td>Open Manhole</td><td>1800</td></tr><tr><td>5.000</td><td>o</td><td>300</td><td>16</td><td>61.400</td><td>59.900</td><td>1.200</td><td>Open Manhole</td><td>1200</td></tr><tr><td>5.001</td><td>o</td><td>300</td><td>17</td><td>61.300</td><td>59.605</td><td>1.395</td><td>Open Manhole</td><td>1200</td></tr><tr><td>5.002</td><td>o</td><td>450</td><td>18</td><td>61.000</td><td>59.191</td><td>1.359</td><td>Open Manhole</td><td>1350</td></tr><tr><td>5.003</td><td>o</td><td>450</td><td>19</td><td>60.800</td><td>59.016</td><td>1.334</td><td>Open Manhole</td><td>1350</td></tr><tr><td>6.000</td><td>o</td><td>300</td><td>20</td><td>60.500</td><td>59.000</td><td>1.200</td><td>Open Manhole</td><td>1200</td></tr><tr><td>6.001</td><td>o</td><td>300</td><td>21</td><td>60.500</td><td>58.907</td><td>1.293</td><td>Open Manhole</td><td>1200</td></tr><tr><td>5.004</td><td>o</td><td>600</td><td>22</td><td>60.400</td><td>58.174</td><td>1.626</td><td>Open Manhole</td><td>1500</td></tr><tr><td>1.011</td><td>o</td><td>750</td><td>23</td><td>60.500</td><td>57.088</td><td>2.662</td><td>Open Manhole</td><td>1800</td></tr><tr><td>7.000</td><td>o</td><td>300</td><td>24</td><td>60.500</td><td>59.000</td><td>1.200</td><td>Open Manhole</td><td>1200</td></tr><tr><td>1.012</td><td>o</td><td>750</td><td>25</td><td>60.000</td><td>57.007</td><td>2.243</td><td>Open Manhole</td><td>1800</td></tr><tr><td>1.013</td><td>o</td><td>750</td><td>26</td><td>59.600</td><td>56.851</td><td>1.999</td><td>Open 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Causeway		Network 2019.1						
PIPELINE SCHEDULES for Storm								
Upstream Manhole								
PN	Hyd Diam Sect (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)	
9.000	o 300	28	60.250	58.750	1.200	Open Manhole	1200	
8.001	o 525	29	60.500	58.085	1.890	Open Manhole	1500	
10.000	o 300	30	60.000	58.500	1.200	Open Manhole	1200	
11.000	o 300	31	60.500	59.000	1.200	Open Manhole	1200	
11.001	o 300	32	60.250	58.750	1.200	Open Manhole	1200	
8.002	o 525	33	59.900	57.574	1.801	Open Manhole	1500	
8.003	o 600	35	59.500	57.006	1.894	Open Manhole	1500	
1.014	o 750	36	59.500	56.743	2.007	Open Manhole	1800	
12.000	o 300	37	59.000	57.500	1.200	Open Manhole	1200	
1.015	o 750	37	59.000	56.615	1.635	Open Manhole	1800	
1.016	o 750	38	56.750	55.200	0.800	Open Manhole	1800	
Downstream Manhole								
PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
9.000	28.500	64.8	29	60.500	58.310	1.890	Open Manhole	1500
8.001	60.300	118.0	33	59.900	57.574	1.801	Open Manhole	1500
10.000	30.200	150.2	33	59.900	58.299	1.301	Open Manhole	1500
11.000	22.800	91.2	32	60.250	58.750	1.200	Open Manhole	1200
11.001	25.600	26.9	33	59.900	57.799	1.801	Open Manhole	1500
8.002	65.800	133.5	35	59.500	57.081	1.894	Open Manhole	1500
8.003	33.900	300.0	36	59.500	56.893	2.007	Open Manhole	1800
1.014	38.300	300.0	37	59.000	56.615	1.635	Open Manhole	1800
12.000	33.000	76.7	37	59.000	57.070	1.630	Open Manhole	1800
1.015	105.200	74.3	38	56.750	55.200	0.800	Open Manhole	1800
1.016	20.000	100.0	41	56.250	55.000	0.500	Open Manhole	1800
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14.000	73.200	63.7	40	57.500	55.700	1.350	Open Manhole	1350																																																																																																																												
13.001	23.000	32.9	41	56.250	55.000	1.025	Open Manhole	1800																																																																																																																												
1.017	12.000	150.0		56.250	54.770	1.255	Open Manhole	0																																																																																																																												
Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)																																																																																																																														
1.017		56.250	54.770	0.000	0	0																																																																																																																														
Volumetric Runoff Coeff	0.950	Additional Flow - % of Total Flow	0.000																																																																																																																																	
Areal Reduction Factor	1.000	MADD Factor * 10m³/ha Storage	10.000																																																																																																																																	
Hot Start (mins)	0	Inlet Coefficient	0.800																																																																																																																																	
Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000																																																																																																																																	
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60																																																																																																																																	
Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1																																																																																																																																	

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<u>Synthetic Rainfall Details</u> Region England and Wales Cv (Summer) 0.950 M5-60 (mm) 26.250 Cv (Winter) 0.840 Ratio R 0.400 Storm Duration (mins) 30 Profile Type Summer		
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Online Controls for Storm

Hydro-Brake® Optimum Manhole: 40, DS/PN: 13.001, Volume (m³): 25.4

Unit Reference	MD-SHE-0196-2100-1500-2100
Design Head (m)	1.500
Design Flow (l/s)	21.0
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	196
Invert Level (m)	55.700
Minimum Outlet Pipe Diameter (mm)	225
Suggested Manhole Diameter (mm)	1500

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.500	21.0
Flush-Flo™	0.454	21.0
Kick-Flo®	0.989	17.2
Mean Flow over Head Range	-	18.1

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	6.8	1.200	18.9	3.000	29.2	7.000	44.0
0.200	18.3	1.400	20.3	3.500	31.5	7.500	45.5
0.300	20.4	1.600	21.6	4.000	33.6	8.000	46.9
0.400	20.9	1.800	22.9	4.500	35.5	8.500	48.3
0.500	21.0	2.000	24.1	5.000	37.4	9.000	49.7
0.600	20.7	2.200	25.2	5.500	39.1	9.500	51.0
0.800	19.8	2.400	26.3	6.000	40.8		
1.000	17.3	2.600	27.3	6.500	42.4		

Hydro-Brake® Optimum Manhole: 41, DS/PN: 1.017, Volume (m³): 12.5

Unit Reference	MD-SHE-0197-2100-1400-2100
Design Head (m)	1.400
Design Flow (l/s)	21.0
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	197
Invert Level (m)	54.850
Minimum Outlet Pipe Diameter (mm)	225
Suggested Manhole Diameter (mm)	1500

DHA Transport Ltd		Page 13																																																																								
Eclipse House Eclipse Park Sittingbourne Road Maidstone ME14 3EN	Pound Farm - Detailed																																																																									
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Causeway		Network 2019.1																										
<u>Storage Structures for Storm</u> <u>Cellular Storage Manhole: 40, DS/PN: 13.001</u> Invert Level (m) 55.700 Safety Factor 2.0 Infiltration Coefficient Base (m/hr) 0.00000 Porosity 0.95 Infiltration Coefficient Side (m/hr) 0.00000 <table border="1"> <thead> <tr> <th>Depth (m)</th> <th>Area (m²)</th> <th>Inf. Area (m²)</th> <th>Depth (m)</th> <th>Area (m²)</th> <th>Inf. Area (m²)</th> </tr> </thead> <tbody> <tr> <td>0.000</td> <td>840.0</td> <td>0.0</td> <td>1.001</td> <td>0.0</td> <td>0.0</td> </tr> <tr> <td>1.000</td> <td>840.0</td> <td>0.0</td> <td></td> <td></td> <td></td> </tr> </tbody> </table> <u>Tank or Pond Manhole: 41, DS/PN: 1.017</u> Invert Level (m) 54.850 <table border="1"> <thead> <tr> <th>Depth (m)</th> <th>Area (m²)</th> <th>Depth (m)</th> <th>Area (m²)</th> </tr> </thead> <tbody> <tr> <td>0.000</td> <td>3171.0</td> <td>1.400</td> <td>4210.0</td> </tr> </tbody> </table>			Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)	0.000	840.0	0.0	1.001	0.0	0.0	1.000	840.0	0.0				Depth (m)	Area (m ²)	Depth (m)	Area (m ²)	0.000	3171.0	1.400	4210.0
Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)																							
0.000	840.0	0.0	1.001	0.0	0.0																							
1.000	840.0	0.0																										
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Pound Farm - Detailed

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Micro
Drainage

1 year Return Period Summary of Critical Results by Maximum Level (Rank 1)
for Storm

Simulation Criteria

Areal Reduction Factor 1.000
Hot Start (mins) 0
Hot Start Level (mm) 0
Manhole Headloss Coeff (Global) 0.500
Foul Sewage per hectare (l/s) 0.000

Additional Flow - % of Total Flow 0.000
MADD Factor * 10m³/ha Storage 10.000
Inlet Coeffiecient 0.800
Flow per Person per Day (l/per/day) 0.000

Number of Input Hydrographs 0
Number of Online Controls 2
Number of Offline Controls 0

Number of Storage Structures 2
Number of Time/Area Diagrams 0
Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FEH D1 (1km) 0.324
FEH Rainfall Version 1999 D2 (1km) 0.385
Site Location D3 (1km) 0.273
C (1km) -0.024 E (1km) 0.310

F (1km) 2.515
Cv (Summer) 0.900
Cv (Winter) 0.900

Margin for Flood Risk Warning (mm) 300.0
Analysis Timestep 2.5 Second Increment (Extended)
DTS Status ON
DVD Status OFF
Inertia Status OFF

Profile(s) Summer and Winter
Duration(s) (mins) 30, 60, 120, 240, 360, 480, 960, 1440
Return Period(s) (years) 1, 30, 100
Climate Change (%) 0, 35, 40

US/MH

PN

Name

Storm

Return Period

Climate Change

First (X) Surcharge

First (Y) Flood

First (Z) Overflow

Overflow Act.

Water Level (m)

1.000

1 30

Summer

1

+0%

100/30 Summer

60.094

1.001

2 30

Summer

1

+0%

30/30 Summer

59.849

1.002

3 30

Summer

1

+0%

30/30 Summer

59.743

1.003

4 30

Summer

1

+0%

100/30 Summer

59.348

1.004

5 30

Summer

1

+0%

100/30 Summer

59.000

1.005

6 30

Summer

1

+0%

100/30 Summer

58.568

2.000

7 30

Summer

1

+0%

100/30 Summer

59.347

1.006

8 30

Summer

1

+0%

100/30 Summer

58.337

3.000

9 30

Summer

1

+0%

100/30 Summer

59.099

3.001

10 30

Summer

1

+0%

100/30 Summer

58.806

1.007

11 30

Summer

1

+0%

30/30 Summer

57.971

4.000

12 30

Summer

1

+0%

100/30 Summer

59.065

1.008

13 30

Summer

1

+0%

30/30 Summer

57.876

1.009

14 30

Summer

1

+0%

30/30 Summer

57.662

1.010

15 30

Summer

1

+0%

30/30 Summer

57.534

5.000

16 30

Summer

1

+0%

100/30 Summer

59.976

5.001

17 30

Summer

1

+0%

30/30 Summer

59.716

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Causeway Network 2019.1

1 year Return Period Summary of Critical Results by Maximum Level (Rank 1)
for Storm

PN	US/MH Name	Surcharged		Flooded		Pipe		Status	Level Exceeded
		Depth (m)	Volume (m³)	Flow / Cap.	Overflow (l/s)	Flow (l/s)			
1.000	1	-0.206	0.000	0.21		17.6		OK	
1.001	2	-0.195	0.000	0.27		20.6		OK	
1.002	3	-0.187	0.000	0.30		26.7		OK	
1.003	4	-0.352	0.000	0.11		32.1		OK	
1.004	5	-0.400	0.000	0.13		46.3		OK	
1.005	6	-0.384	0.000	0.16		49.6		OK	
2.000	7	-0.253	0.000	0.06		9.4		OK	
1.006	8	-0.457	0.000	0.13		61.9		OK	
3.000	9	-0.201	0.000	0.24		18.1		OK	
3.001	10	-0.349	0.000	0.12		27.4		OK	
1.007	11	-0.553	0.000	0.16		93.8		OK	
4.000	12	-0.235	0.000	0.11		15.0		OK	
1.008	13	-0.544	0.000	0.17		117.8		OK	
1.009	14	-0.521	0.000	0.20		122.1		OK	
1.010	15	-0.546	0.000	0.17		128.3		OK	
5.000	16	-0.224	0.000	0.14		14.8		OK	
5.001	17	-0.189	0.000	0.29		24.6		OK	

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1 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Storm									

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Pound Farm - Detailed

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Micro Drainage

30 year Return Period Summary of Critical Results by Maximum Level (Rank 1)for Storm

Simulation Criteria

Areal Reduction Factor 1.000Additional Flow - % of Total Flow 0.000

Hot Start (mins) 0MADD Factor * 10m³/ha Storage 10.000

Hot Start Level (mm) 0Inlet Coeffiecient 0.800

Manhole Headloss Coeff (Global) 0.500Flow per Person per Day (l/per/day) 0.000

Foul Sewage per hectare (l/s) 0.000

Number of Input Hydrographs 0Number of Storage Structures 2

Number of Online Controls 2Number of Time/Area Diagrams 0

Number of Offline Controls 0Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FEH D1 (1km) 0.324F (1km) 2.515

FEH Rainfall Version 1999 D2 (1km) 0.385Cv (Summer) 0.900

Site Location D3 (1km) 0.273Cv (Winter) 0.900

C (1km) -0.024E (1km) 0.310

Margin for Flood Risk Warning (mm)300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS StatusON

DVD StatusOFF

Inertia StatusOFF

Profile(s)Summer and Winter

Duration(s) (mins) 30, 60, 120, 240, 360, 480, 960, 1440

Return Period(s) (years)1, 30, 100

Climate Change (%)0, 35, 40

	US/MH		Return	Climate	First (X)	First (Y)	First (Z)	Overflow	Water
PN	Name	Storm	Period	Change	Surcharge	Flood	Overflow	Act.	Level
1.000	1 30	Summer	30	+35%	100/30	Summer			60.264
1.001	2 30	Summer	30	+35%	30/30	Summer			60.110
1.002	3 30	Summer	30	+35%	30/30	Summer			59.999
1.003	4 30	Summer	30	+35%	100/30	Summer			59.454
1.004	5 30	Summer	30	+35%	100/30	Summer			59.155
1.005	6 30	Summer	30	+35%	100/30	Summer			58.750
2.000	7 30	Summer	30	+35%	100/30	Summer			59.398
1.006	8 30	Summer	30	+35%	100/30	Summer			58.644
3.000	9 30	Summer	30	+35%	100/30	Summer			59.238
3.001	10 30	Summer	30	+35%	100/30	Summer			58.934
1.007	11 30	Summer	30	+35%	30/30	Summer			58.597
4.000	12 30	Summer	30	+35%	100/30	Summer			59.139
1.008	13 30	Summer	30	+35%	30/30	Summer			58.562
1.009	14 30	Summer	30	+35%	30/30	Summer			58.502
1.010	15 30	Summer	30	+35%	30/30	Summer			58.458
5.000	16 30	Summer	30	+35%	100/30	Summer			60.103
5.001	17 30	Summer	30	+35%	30/30	Summer			60.014

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Causeway Network 2019.1

30 year Return Period Summary of Critical Results by Maximum Level (Rank 1)
for Storm

PN	US/MH Name	Surcharged	Flooded	Flow / Overflow		Pipe	Status	Level Exceeded
		Depth (m)	Volume (m³)	Cap.	(l/s)	Flow (l/s)		
1.000	1	-0.036	0.000	0.82		68.6	OK	
1.001	2	0.066	0.000	1.00		77.6	SURCHARGED	
1.002	3	0.069	0.000	1.15		102.0	SURCHARGED	
1.003	4	-0.246	0.000	0.41		124.7	OK	
1.004	5	-0.245	0.000	0.54		194.5	OK	
1.005	6	-0.202	0.000	0.69		210.2	OK	
2.000	7	-0.202	0.000	0.23		38.5	OK	
1.006	8	-0.150	0.000	0.56		266.6	OK	
3.000	9	-0.062	0.000	0.97		73.6	OK	
3.001	10	-0.221	0.000	0.50		117.2	OK	
1.007	11	0.073	0.000	0.62		372.6	SURCHARGED	
4.000	12	-0.161	0.000	0.43		60.9	OK	
1.008	13	0.142	0.000	0.65		455.5	SURCHARGED	
1.009	14	0.319	0.000	0.66		397.1	SURCHARGED	
1.010	15	0.378	0.000	0.55		425.3	SURCHARGED	
5.000	16	-0.097	0.000	0.57		59.1	OK	
5.001	17	0.109	0.000	1.19		100.5	SURCHARGED	